

## City of Charleston

# Board of Architectural Review-LARGE September 25, 2019

DEPARTMENT OF PLANNING, PRESERVATION & SUSTAINABILITY

Agenda Item 1:

Approval of minutes from the March 13, 2019 regular meeting.

Agenda Item 2:

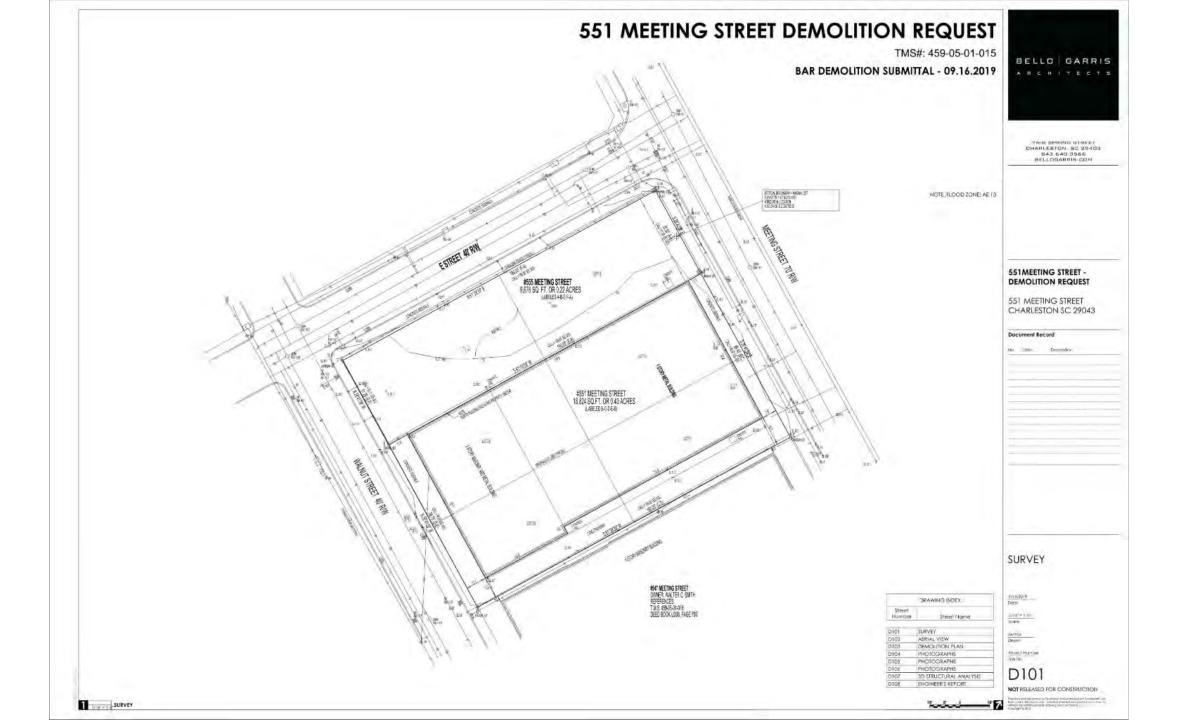
Approval of minutes from the March 13, 2019 special meeting.

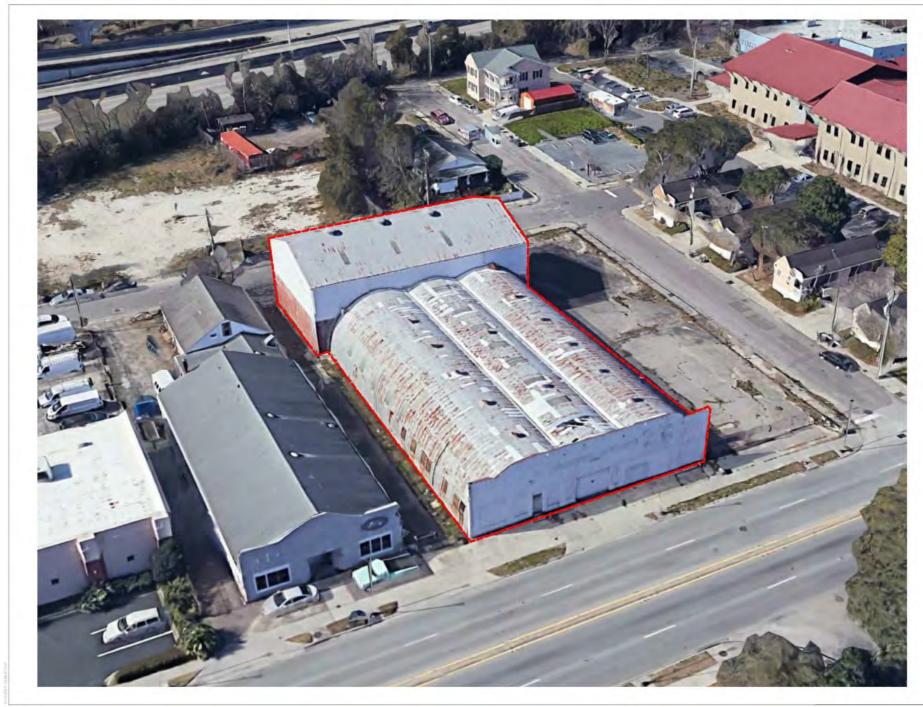
Agenda Item 3:

551 Meeting Street - - TMS # 459-05-01-015

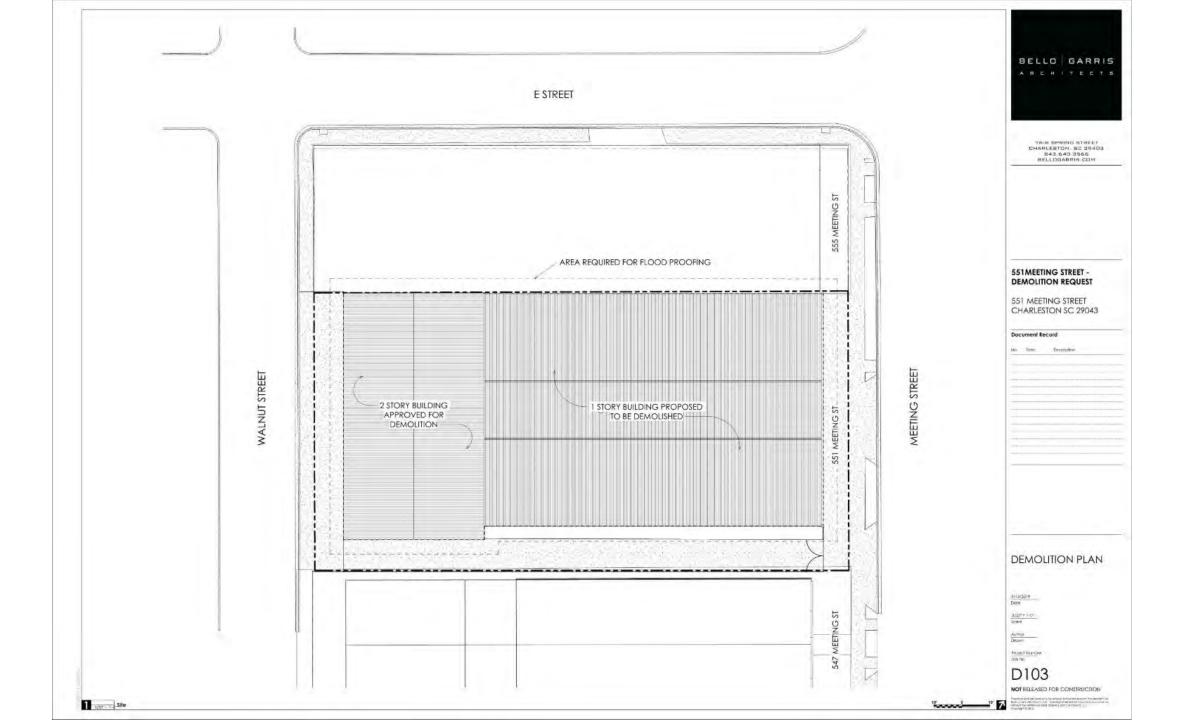
Request approval for demolition of existing structure.

Not Rated / (East Side) / c. 1944-51; 1951-55 / Historic Corridor District

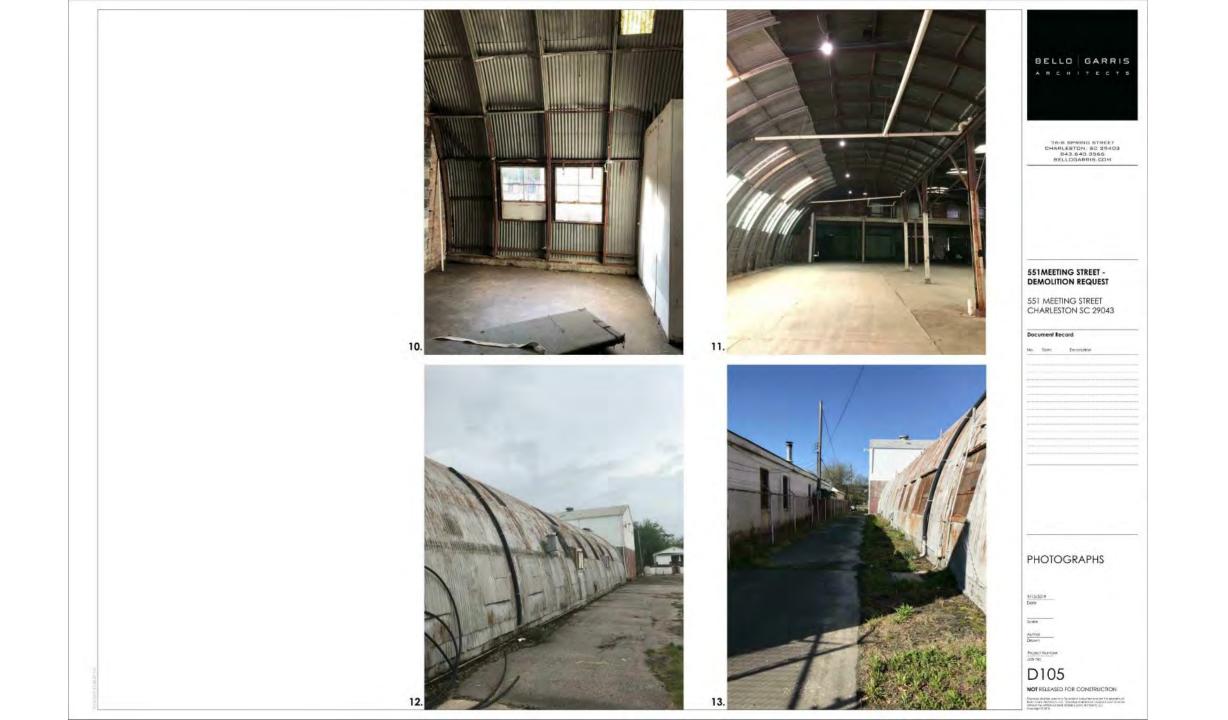


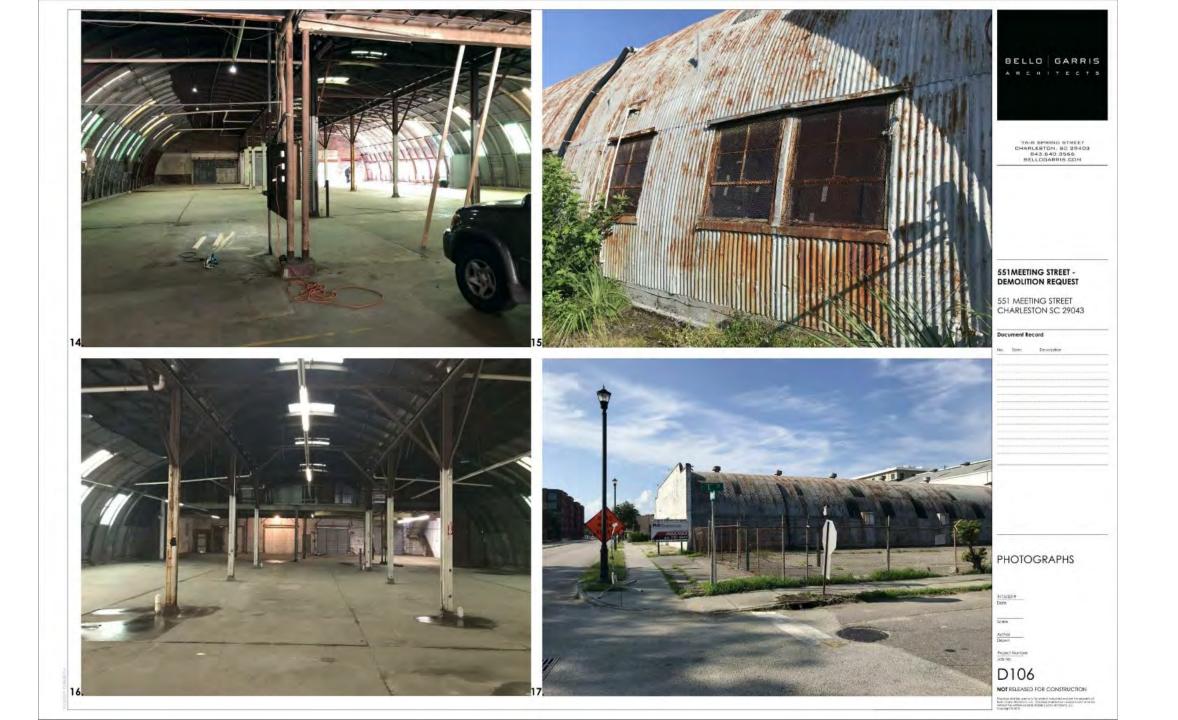


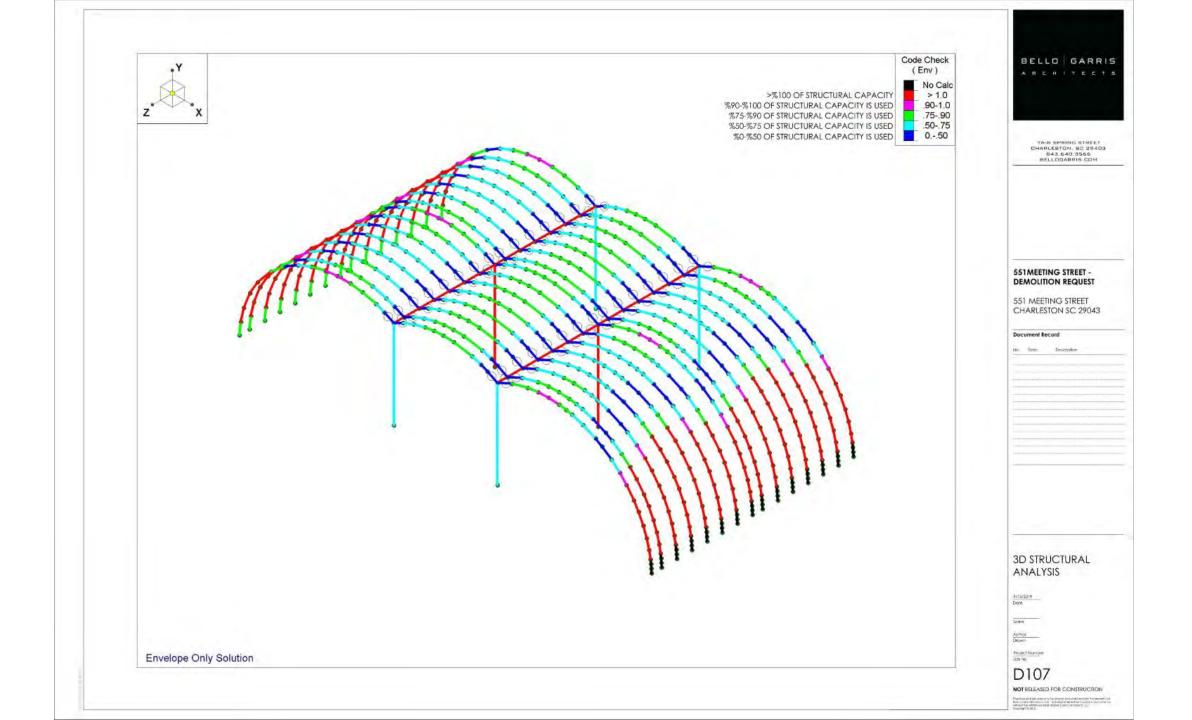
BELLO GARRIS ARCHITECTS 76-B SPRING STREET CHARLESTON, BC 29403 843,640.3566 BELLDGARRIB.COM 551 MEETING STREET -DEMOLITION REQUEST 551 MEETING STREET CHARLESTON SC 29043 Document Record No, Date Description AERIAL VIEW 9/13/2019 Dore 3/4" + 1"-0" Scale Author Drawn Project Number Job No. D102 NOT RELEASED FOR CONSTRUCTION Develops double- used andy for project indication shat so the property of indications functions. U.C. Dissurds, that not be copied in part or whole whole the writes conserve of balls ( parts worthout a SC Dissurds) (0.214).













September 13, 2019

551 Meeting Street, LLC Attn: Mr. Ross Cowan 518 East Bay St. Charleston, SC 29403

RE: Structural Analysis of Existing Framing 551 Meeting St. Charleston, SC Tobias & West, LLC Project # 18-078

Dear Mr. Cowan:

Per our discussion, I understand that a change in occupancy is proposed for the existing building at the above referenced address. This will require the entire building to be brought up to meet the current 2015 International Building Code (IBC), including wind and seismic design loads. This requires all structural members (framing, foundation, sheathing, roof coverings, connections) to be analyzed for their ability to meet the current design loads specified in the 2015 IBC.

The property is also located in a flood zone with the ground floor slab set approximately 3'-0" below the FEMA base slab elevation. Any improvements that increase the value of the building more than 50% of the value of the existing structure will require it to conform to ASCE 24-14 and FEMA design requirements. Since the usable space is below the flood elevation, this will require "dry flood-proofing" installed to at least 1'-0" above the FEMA base flood elevation. This is normally achieved through manufacturer-specific flood barriers that can be dropped into place on a separate foundation around the building.

#### Analysis:

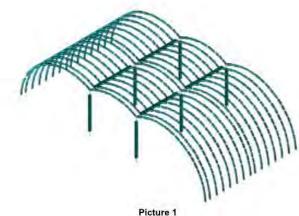
As previously stated, the existing structure must be analyzed for the current 2015 IBC design loads. This was accomplished by modeling it utilizing a current version of RISA 3D analysis software (See Picture 1). Load combinations as established in the 2015 IBC were used in the analysis. No information is known regarding the existing foundations, but we assume they are isolated, shallow footings.

Below are the design loads used for our analysis:

| Dead Load =      | 10 psf                              |
|------------------|-------------------------------------|
| Roof Live Load = | 20 psf                              |
| Wind Load =      | Loads per 147 mph – Exposure B      |
| Seismic Load =   | Loads per Seismic Design Category D |
| Snow Load =      | 3 psf                               |

- Tobias & West, LLC 1514 Mathis Ferry Rd., Suite 216 (Mailing: PO Box 887) Mt. Pleasant, SC 29464 (29465)
- charleston: 843.216.9820 columbia: 803.955.6464 www.tobiaswest.com

- Charleston - Columbia -



Ficture 1

There are three main member types that our analysis focused on for this review: arched rafters (Ribs), main support beams (Girders) and the Columns. These members are all steel, cold-rolled shapes and consist of the following:

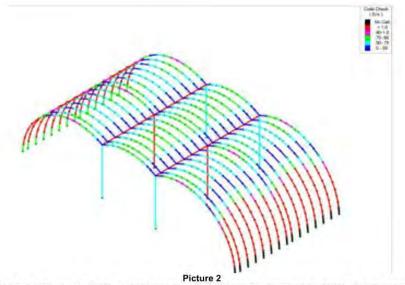
Ribs: Built-up I-shape with dimensions 4" wide x 6" deep (approximately 3/32" thick) Beams & Columns: Built-up I-shape with dimensions 8" wide x 8" deep (approximately 1/8" thick)

We estimated a steel strength of 30 KSI for these members. This is based upon the age of the structure and the cold-rolled forming process. New buildings of this type utilize 50 KSI and 80 KSI steel. The existing member types are similar to a 12 gauge metal stud that can be found in commercial building walls. In addition, the roof consists of corrugated metal roofing with corrosion noted throughout the roof, with the worst-case corrosion occurring at the roof bearing location at grade. It is supported on 2"x2" wood purlins spanning between the Ribs. The shear resistance of the corrugated roofing for anchorage to the main structure is not known.

#### Results:

The design software utilized analyzes all the members and reports results in different formats. One particular way to view the results is with the program's overstress results as shown in Picture 2. This is a screenshot showing these results. The areas in red are overstressed or failing. The areas in Magenta are 90% - 100% stressed.

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The Ribs are stressed by 150% of their capacity. The Girders are stressed by 400% of their capacity and the columns are stressed by 110%. Note that the columns at the ends do not appear overstressed, but this is due to the fact that they are not carrying the full design load. The columns in the middle are a true representation of the analysis.

As previously stated, the existing foundation is unknown. But, given that the existing building was intended to be temporary, it is safe to assume the existing footings (if any) will be insufficient to resist current wind or seismic loads. Therefore, we expect that the existing footings will need to be increased. Our calculations show that an 8'x8'x1' deep footing minimum is required below the main columns. In addition, since the existing slab is most likely unreinforced or under reinforced, it will need to be removed and replaced with a slab designed for the buoyancy pressure created from the dry flood-proofing condition.

551 Meeting Street – Charleston, SC – September 13, 2019 Pg. 3 of 6 – *Tobias & West, LLC*  The lateral resistance to wind and seismic loads is achieved by use of steel angles serving as knee braces on the columns along both column lines (See Picture 3). These type connections are not approved for high-seismic locations. I recommend removing the columns in 8 locations and replacing with steel moment frames that meet the current seismic design standards. See Pictures 4 and 5 for the anticipated frame locations and a typical moment frame elevation.



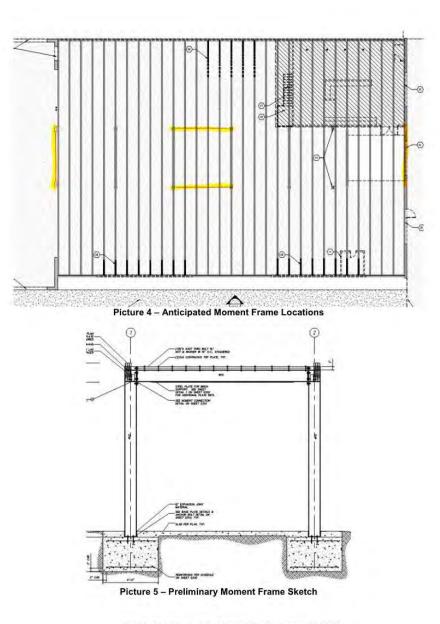
Picture 3 -Knee braces at columns

The front entry wall of the building is unreinforced masonry block and will be insufficient to resist seismic or wind lateral loads. This wall must be removed and replaced with either a new properly designed masonry shear wall or a steel moment frame. The anticipated steel moment frame location is shown in Picture 4 on the next page. New footings will be required at the moment frame column locations.

The rear of the building does not currently have any lateral resisting elements. This location will require either a new properly designed masonry shear wall or a steel moment frame similar to the front entry wall. New footings below the columns will be required in this location as well.

A geotechnical investigation and addendum was performed for this site (Attached). These reports explain that the soil conditions are marginal at best and become worse towards the rear portion of the site. It is highly likely that the new footings at the rear of the structure will need to be pilesupported. The report also states "We recommend that existing foundations be evaluated for settlement and distress due to underlying debris fill and soft soils; underpinning with helical piers may be required." "Shallow foundations should not bear on existing debris fill". So, there are unknown soil conditions present below the existing foundations that may require additional support (i.e. helical piles, etc.).

> 551 Meeting Street – Charleston, SC – September 13, 2019 Pg. 4 of 6 – Tobias & West, LLC



551 Meeting Street – Charleston, SC – September 13, 2019 Pg. 5 of 6 – *Tobias & West, LLC* 

#### Conclusions:

The analysis of the structural framing has shown that the current steel members do not have the capacity to support current 2015 IBC design loads. An additional analysis of the existing framing was performed using the same design software and the cold-rolled Rib sizes should be a minimum of 10" deep to provide sufficient support for the roof system. Therefore, I recommend removing and replacing the Ribs with properly designed light-gauge or hot-rolled steel shapes. The steel Ribs should be galvanized since they will be below the FEMA base flood elevation.

The analysis also demonstrated that the Girders would be overstressed and deflect more than 4" under the design loads. The Columns are overstressed with only vertical loads applied to them. Several of the columns and beams must be removed for the addition of steel moment frames for lateral resistance. The remaining Girders must be replaced with steel wide-flange beams and the columns must also be replaced with either steel wide flange or tube steel columns. The new steel columns should be galvanized since they will be below the FEMA base flood elevation. This means all Girders and Columns will be removed and replaced.

The existing corrugated roof covering must be removed and replaced with a material that has sufficient shear resistance to transfer lateral loads into the framing. It should also be galvanized since a portion of it will be below the base flood elevation. Additional supports must be installed at intervals to provide support of the roofing between the Ribs. Those members could most likely be light-gauge similar to the Ribs.

Finally, based upon review of the geotechnical reports, the existing soil conditions are poor consisting of considerable debris fill. Piling or helical piles may be required at the existing footings and will most likely be required for new footings at the rear of the site. The extent of the below-grade work will not be known until excavation is completed for the new foundations.

This report is intended for the sole use of *551 Meeting Street, LLC* and her successors and assigns upon each such entity's written acceptance of the terms and conditions of the agreement. Reliance on this report is governed by the terms and conditions of our proposal and the agreement under which this work was performed. If other parties wish to rely on this report, a mutual agreement between Tobias & West, LLC and such third party of the terms and conditions for our services can be established prior to their reliance on this information.

Thank you for this opportunity to be of service!

Sincerely,



saw - attachments

551 Meeting Street – Charleston, SC – September 13, 2019 Pg. 6 of 6 – *Tobias & West, LLC* 

Agenda Item 4:

## 547 Meeting Street - - TMS # 459-05-01-016

Request approval for demolition of existing structure.

Not Rated / (none) / c. 1944-51 / Historic Corridor District



## **Demolition Request**

Requesting permission for full demolition of the structure at 547 Meeting Street. At the August 14th BAR Large Meeting the Demolition Request was deferred. The BAR and City Staff requested that the applicant perform a code analysis to determine what would be required for the building to be upgraded to meet the currrent building code.

Dan Martin, PE of Britt, Peters and Associates conducted a Tier 1 structural code analysis of the bulding. The analysis concluded that there are several deficiencies within the building that would need to be adddressed for the building to meet current code.

The applicant believes these modifications to be significant alterations to the buildings's currrent character and therefore demolition is justified.

#### Staff Comments from August 14th Meeting:

1. This is a similiar application to the buildings at 511 Meeting STreet, the 1-story Constantine facade at 577 Meeting Street, and the Quonset Hut directly north of this building, that also requested total demolition. All three of those structures were required to be saved, having historic value and/or representing unique structures in Charleston.

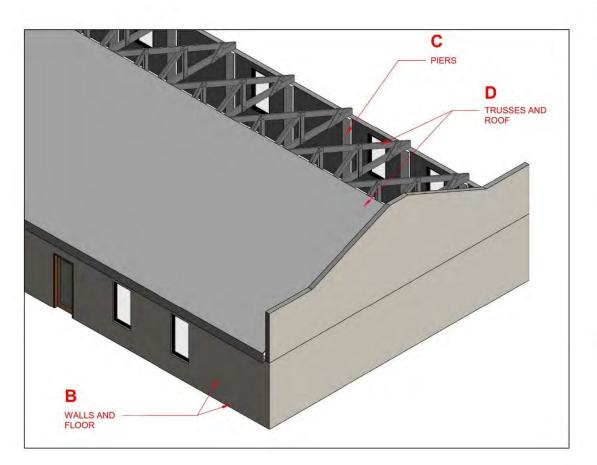
2. Staff is comfortable with the demolition of the rear post-1951 structures as they primarily consist of inferior materials. On the other hand, the front building consists of terracotta wall construction, handsome wood truss and plank roof construction supported by brick piers and likely original windows. The building form is also a consideration. We are also comfortable with the removal of the front facade as it appears as a later addition that is incompatible with the building and in significant disrepair.

3. The one issue that could justify demolition relates to the extent of Code upgrades required in a renovation of the front structure that may essentially compromise its current character. This should be studied by the design team and presented with resubmission of the demolition application.

4. If the character of the existing building can be retained as part of a renovation, staff believes the main building could be retained and incorporated into a development given the 8-story height district at the rear of the site.

Deferral for Code study of the exiting building as noted.





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- **B** DIAGRAMS WALLS AND FLOOR
- C DIAGRAMS PIERS
- D DIAGRAMS TRUSSES AND ROOF

## **APPENDIX - SUPPORTING MATERIALS**

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- A1 STRUCTURAL REPORT
- A2 TERRACOTTA ARTICLE
- A3 DRAWINGS
- A4 ORIGINAL BAR SUBMISSION





#### 7.0 FINDINGS AND DEFICENCIES

Based on the Tier 1 Evaluation presented in this report and our site observations, there are several structural deficiencies in this building. The deficiencies indicated would need to be addressed in order for the building to be in compliance with current building codes and standards.

The following is a summary of additional structural deficiencies in this building that may lead to life-safety issues:

- The building does not demonstrate a complete well-defined load path as it relates to the diaphragm. The (4) major concerns are:
  - a. Diaphragm connection to the wall to transfer loads to shear walls.
    - The hollow clay tile would need to have a layer of shotcrete added that would allow for the transfer of forces from the roof diaphragm to the shear walls. Additional wood blocking would need to be added at the top of the wall to facilitate the transfer of forces.
  - b. The horizontal lumber diaphragm does not provide an adequate diaphragm strength.
    - This diaphragm deficiency could be addressed by adding structural sheathing over the horizontal lumber to provide the required strength. This would require the removal of the existing roof covering, placement of the new sheathing, and roof replacement.
  - c. Anchoring of wall for out of plane forces.
    - i. Anchoring of the wall to the diaphragm using the hollow clay tile would require the addition of a member capable of transferring the loads across the wall to the pilasters. This would best be achieved by the application of a shotcrete layer to be adhered to the inside face of the hollow clay tile designed to transfer the out of plane loads to new pilasters capable of resisting the imposed loads.
  - Proportions of the wall indicate a buckling concern for the lateral resisting walls due to in-plane and out of plane loading.
    - The application of a shotcrete layer adhered to the inside face of the hollow clay tile wall would allow for the wall to resist the in-plane and out of plane design forces.
- The hollow clay tile does not provide adequate section to prevent buckling during seismic event and needs to be addressed.
  - a. The application of a shotcrete layer adhered to the inside face of the hollow clay tile wall would allow for the wall to resist the in-plane and out of plane design forces.
- The North gable end wall demonstrates out of plane rotation that needs to be addressed by bracing the joint between the wood framing and wall.
  - Additional diagonal bracing would need to be added to the top of the existing hollow clay tile wall to the adjacent truss members would create an adequate load path.
- 4. The South gable end wall does not have bracing to support the gable for out of plane loads.

1100 Queensborough Blvd, Suite 202, Mount Pleasant, SC 29464 ph. 843/284:0400 www.brittpeters.com fax 843/284/0401

- a. Diagonal bracing similar to the bracing on the north gable wall could need to be added to the top of the existing hollow clay tile wall to the adjacent truss members to create an adequate load path.
- Analysis of the truss indicates that several web members would need to be braced to be considered compression elements. Additionally, several of the bolted and nailed joints are not sufficient to resist the anticipated loading. These joints would need to be reinforced to meet the current design specifications.
  - Blocking would need to be added between the web members to lower the unbraced length
    of the individual members and allow for their use.
  - b. The overloaded joints would need to be reinforced with additional fasteners to resist the design loads.
- 6. The trusses are not adequately tied to the pier elements and the pier elements consist of interlocking solid clay bricks and hollow clay tile element. The system would need to be replaced with a new column element that would ensure a proper load path.
  - a. The truss would need to be properly tied to new pier elements capable of resisting the anticipated loads. The connection would likely consist of steel anchors embedded in the pier and attached to the existing truss with nails or screws.
- The building's floor elevation is under the listed flood elevation and dry floodproofing would be required. The existing wall system would not be able to resist these loads.
  - a. The application of shotcrete to the inside of the hollow clay tile wall would provide for sufficient out of plane wall loads due to flood loads. Additional consideration would need to be given to the support of the flood panels used at the doors and window panels.
  - b. Due to the presence of elevated hydrostatic forces on the dry floodproofing system, the slab will need to be removed and thickened to resist the buoyant forces created by differences in water levels between the inside and outside of the building.

#### 8.0 CONCLUSIONS

In this evaluation, we have provided a single tiered process (Tier 1) for the evaluation of the referenced building, which is located in a region of high seismicity and was evaluated to the Life Safety Performance Level per the provisions of ASCE 41-13. ASCE 41-13 is a referenced standard in the International Existing Building Code. The major emphasis of this report was to investigate the "as-built" condition of the lateral-force resisting system. In general, we have determined that portions of this building do not comply with current seismic safety requirements.

We have also identified several deficiencies with the structural system. Significant structural improvements would be needed to bring the building into compliance with the current building codes and standards.

This concludes our structural evaluation please contact our office if you have any questions or comments.

Sincerely,

Britt, Peters & Associates, Inc.

I dui F. m

Dan Martin, PE (SC #28668) Project Engineer

> 1100 Queensborough Blvd, Suite 202, Mount Pleasant, SC 29464 ph. 843.284.0400 www.brittpeters.com fax 843.284.0401



SUMMARY OF STRUCTURAL FINDINGS | D-34

11

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547 MEETING STREET SEPTEMBER 2, 2019

10



#### PHOTO 1 - PHOTO OF TERRACOTTA BLOCK AT 547 MEETING

PHOTO 2 - PHOTO OF TERRACOTTA BLOCK AT 547 MEETING

### WALLS AND FLOOR SUMMARY

- The exterior walls of the structure at 547 Meeting Street are constructed from terracotta tile blocks.
- According to the article 'Garbage or Gold?, clay tile was used during the early to mid 1900's because it was inexpensive, quick to assemble and resistant to fire. See appendix A2.
- Unfortunately the hollow cores of the block run horizontal, making it impossible for internal reinforcement. This makes the wall more susceptible to buckling during seismic, high wind or flood events and does <u>not</u> <u>meet current code</u>. For that reason, the product is no longer used in modern construction.
- In adition, hollow clay tiles are brittle and easy to shatter.
- Possible solutions are to add a layer of reinforcing to the face of the blocks or to build a separate wall such that the blocks themselves act as a veneer.
- The existing floor slab will need to be removed and replaced with a thicker reinforced slab to resist
  pressures from flood.

PHOTO 3 - PHOTO OF A TYPICAL TERRACOTTA BLOCK FOUND ONLINE

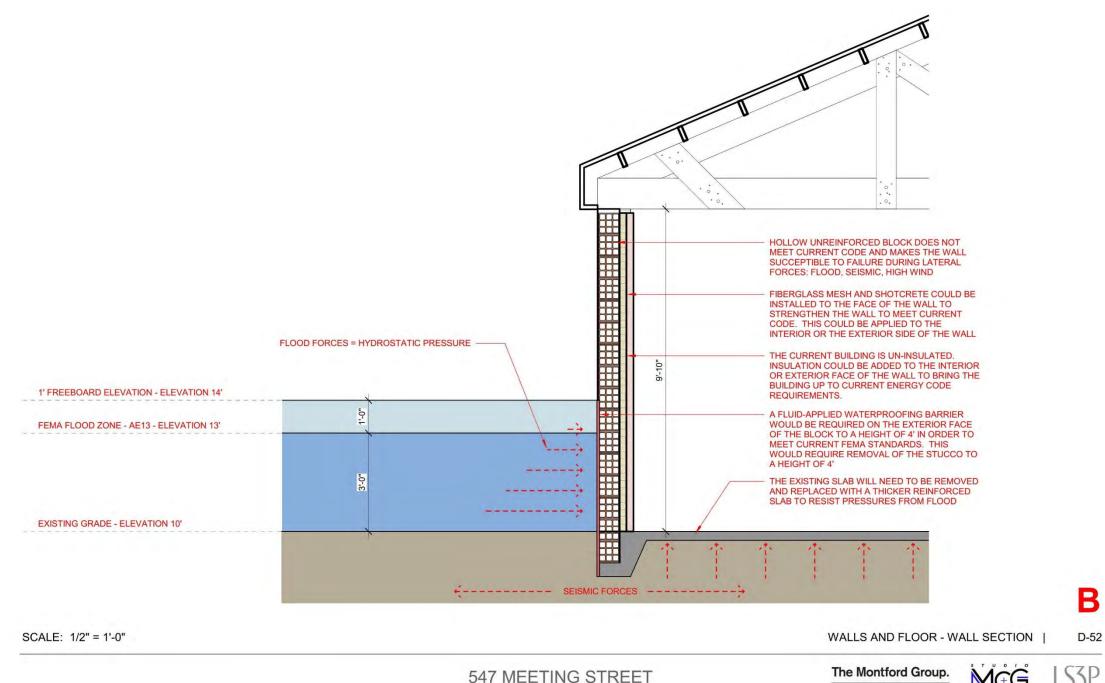
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D-51

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WALLS AND FLOOR - SUMMARY

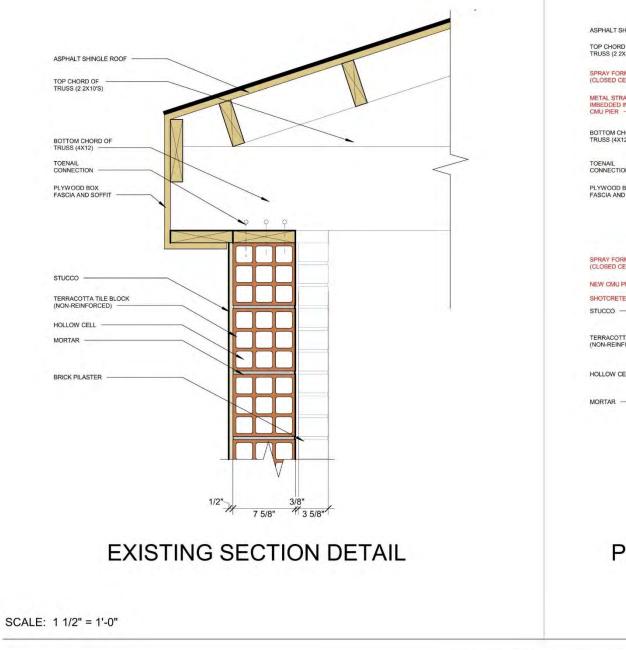


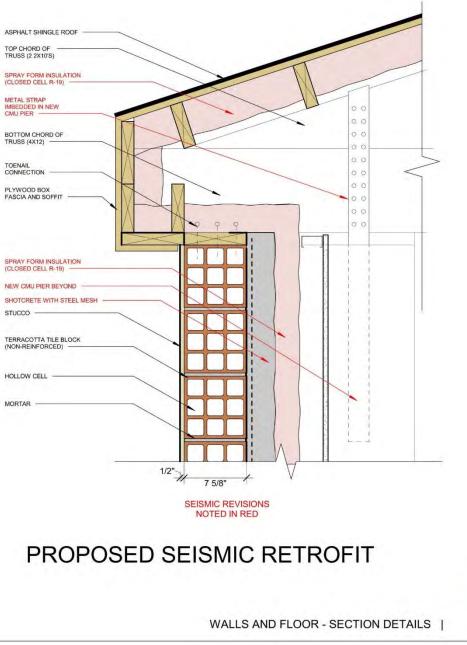


SEPTEMBER 2, 2019

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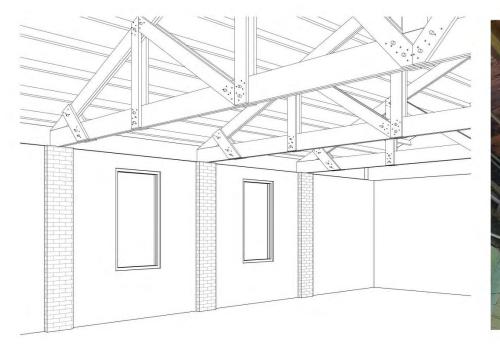
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B

D-53

LS3P

547 MEETING STREET SEPTEMBER 2, 2019





### PIER SUMMARY SUMMARY

- The wooden trusses sit on top of brick and terracotta block piers, spaced 10' on center.
- The piers are un-reinforced and incable of being internally reinforced. In addition the trusses are not properly strapped to the piers.
- These conditions makes the wall more susceptible to buckling during seismic, high wind or flood events and does <u>not meet current code</u>
- Any retrofit would require a new column be built adjacent to the existing pier.

PIER SUMMARY | D-61

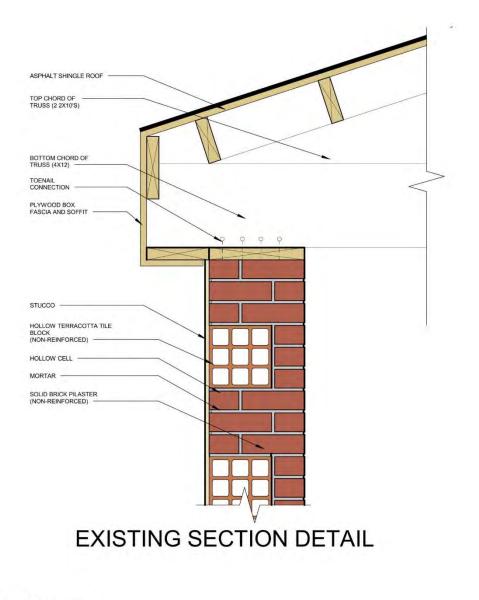
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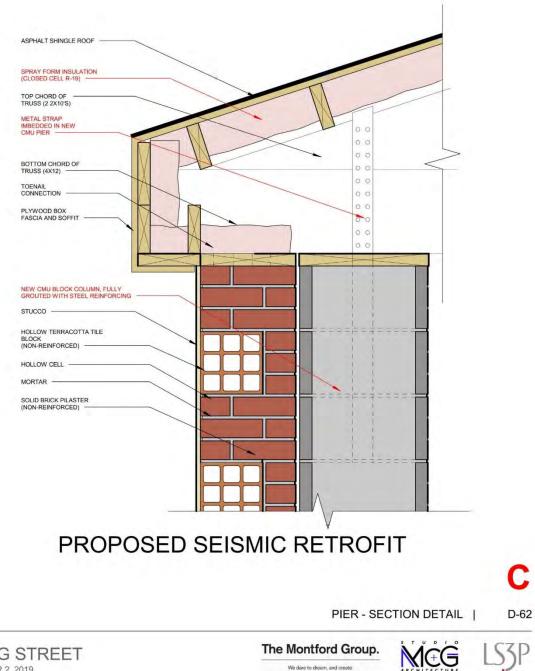


С

547 MEETING STREET SEPTEMBER 2, 2019

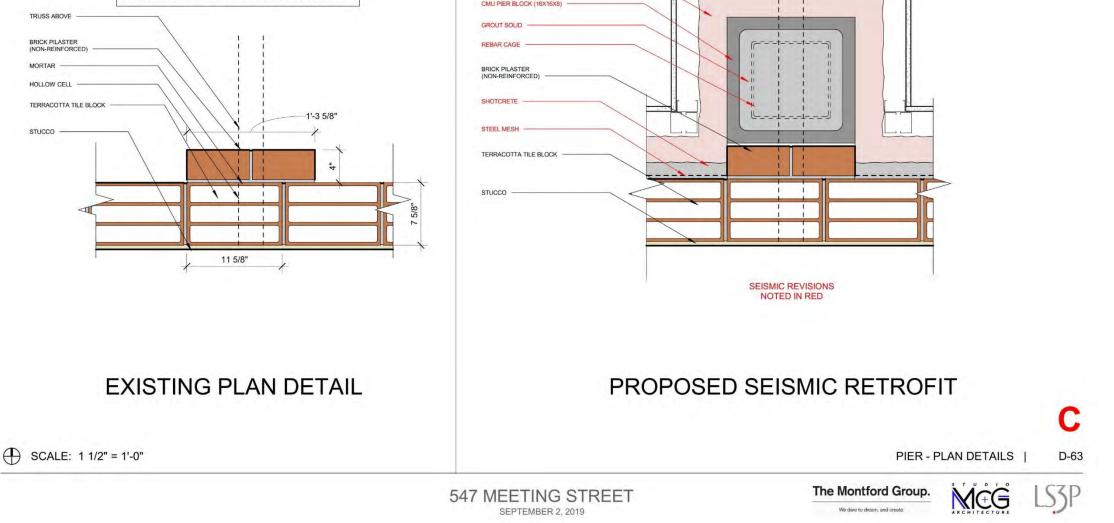
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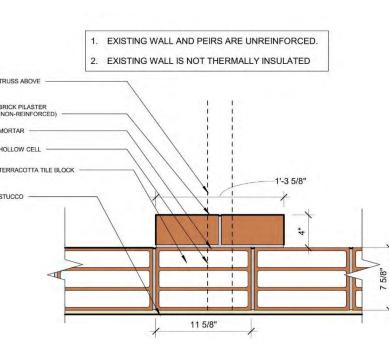


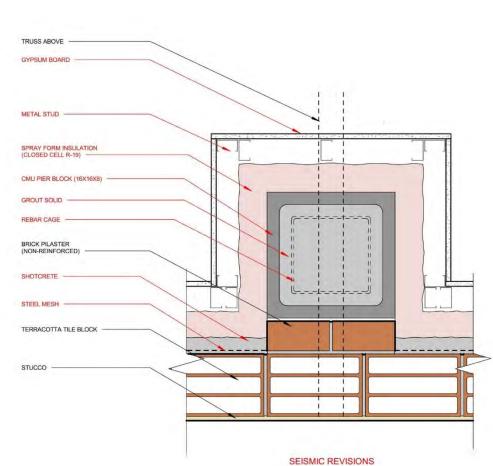


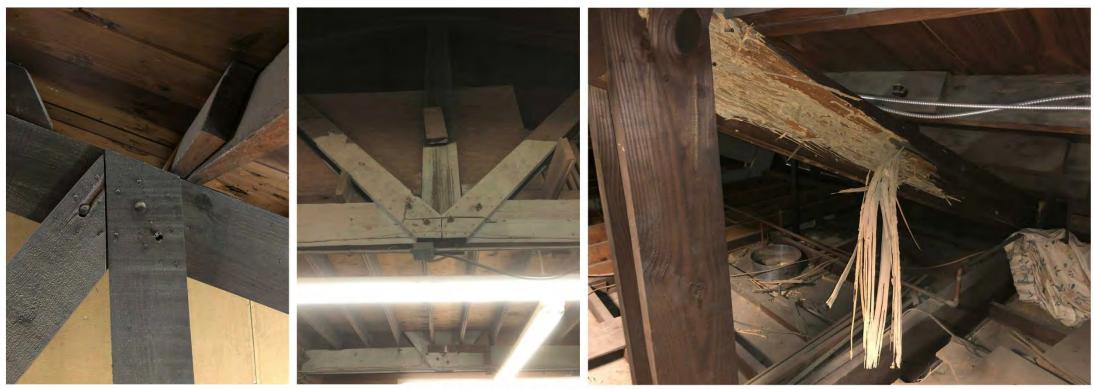
SCALE: 1 1/2" = 1'-0"

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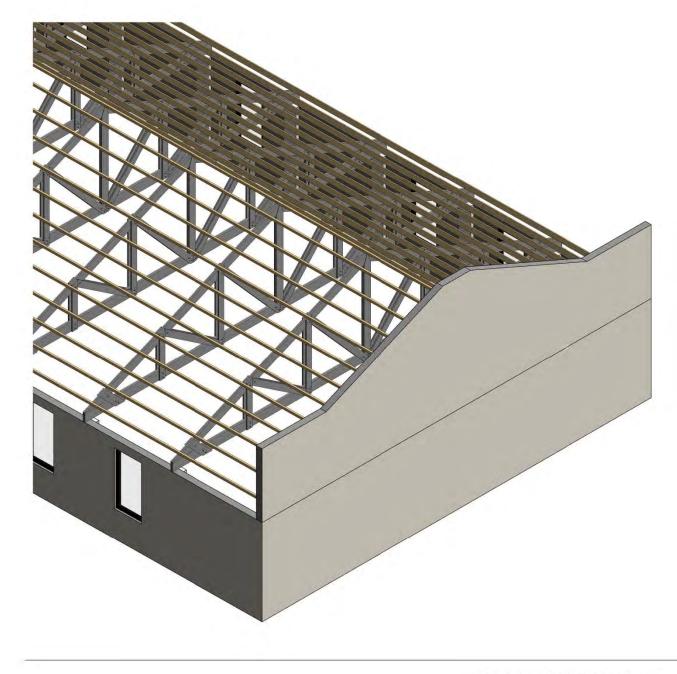
### TRUSS AND ROOF SUMMARY

- The roof is supported by wooden trusses that clear span the building and are placed 10' on center. The trusses are classified as 'stick-built' construction becuase they are comprised of nominal lumber, nailed and bolted together in the field.
- Some of the truss elements require additional bracing due to their size and many of the connections are not sufficient for current code.
- · The trusses are not properly attached to the piers.
- In addition, termite damage was seen in many areas and in some cases the damage was severe. In other areas there was evidence of new lumber that had been added or substituted for damaged members.
- Any retrofit would require that all connections be properly upgraded and damaged lumber be removed or sistered with new lumber.
- The existing plank roofing does not provide adequate diaphram strength. A layer of structural sheathing would need to be added on top of existing planks.
- WOOD TRUSS SUMMARY | D-71

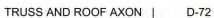




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- The roof is supported by wooden trusses that clear span the building and are placed 10' on center. The trusses are classified as 'stick-built' construction becuase they are comprised of nominal lumber, nailed and bolted together in the field.
- Some of the truss elements require additional bracing due to their size and many of the connections are not sufficient for current code.
- · The trusses are not properly attached to the piers.
- In addition, termite damage was seen in many areas and in some cases the damage was severe. In other areas there was evidence of new lumber that had been added or substituted for damaged members.
- Any retrofit would require that all connections be properly upgraded and damaged lumber be removed or sistered with new lumber.
- The existing plank roofing does not provide adequate diaphram strength. A layer of structural sheathing would need to be added on top of existing planks.

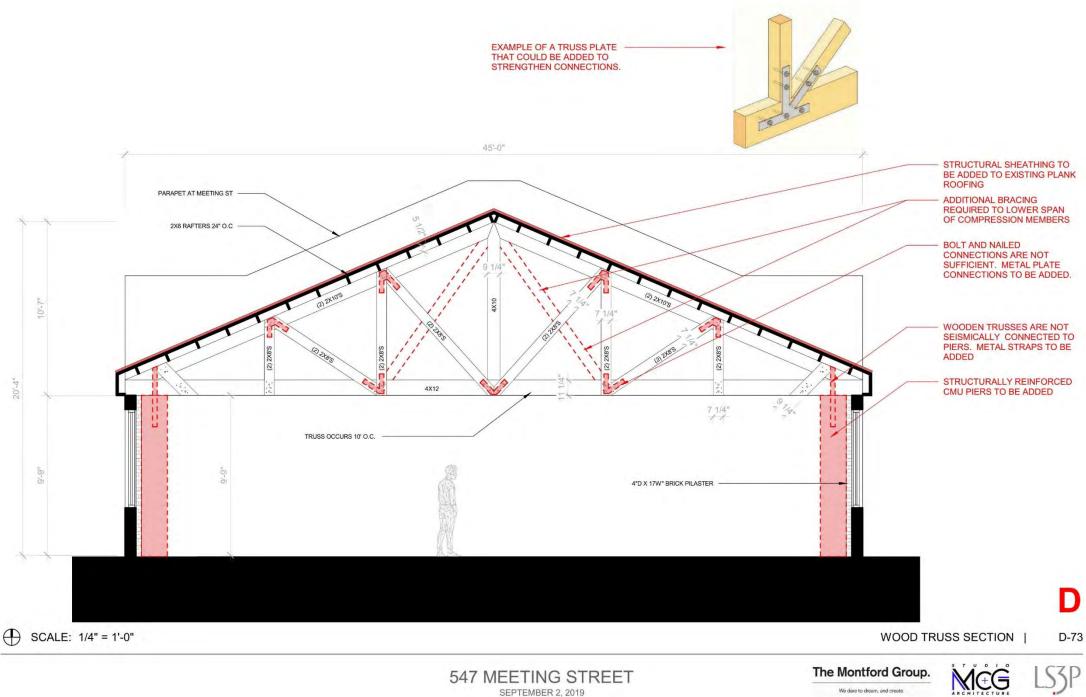


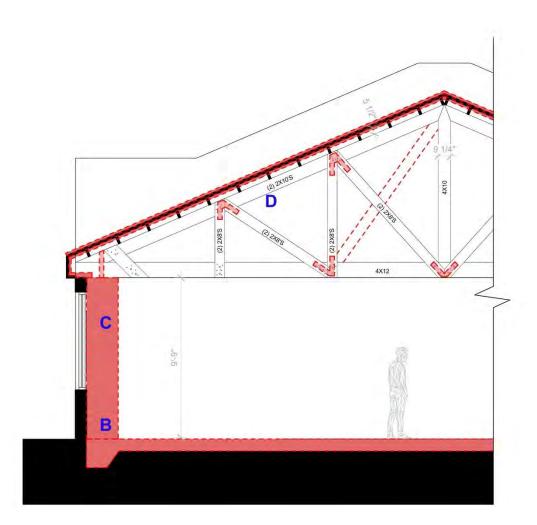
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547 MEETING STREET SEPTEMBER 2, 2019





### **EXECUTIVE SUMMARY**

The building contains deficiencies that would require renovations for the building to meet current code standards. The following is a summary of those deficiencies and changes. For a full report, see the *A1* - *structural report* 

## WALLS AND FLOOR

The exterior walls of the structure at 547 Meeting Street are constructed from terracotta tile blocks. Unfortunately the hollow cores of the blocks run horizontal, making it impossible for internal reinforcement. These conditions makes the wall more susceptible to buckling during seismic, high wind and flood events and does not meet current code. Possible solutions are to add a layer of reinforcing to the face of the blocks or to build a separate wall such that the blocks themselves act as a veneer. The existing floor slab will need to be removed and replaced with a thicker reinforced slab to resist pressures from flood. See D51, 52 and 53 for additional details.

## PIERS

B

C

D

The wooden trusses sit on top of brick and terracotta block piers. The piers are unreinforced and their construction makes it impossible for internal reinforcment. In addition the trusses are not properly strapped to the piers. These conditions makes the wall more susceptible to buckling during seismic, high wind and flood events and does <u>not meet current code</u>. Any retrofit would require new columns be built adjacent to the existing piers. *See D61, 62 and 63 for additional details*.

## **TRUSSES AND ROOF**

The roof is supported by wooden trusses that clear span the building and are placed 10' on center. Some of the truss elements require additional bracing due to their size and many of the connections are not sufficient for current code. The trusses are not properly attached to the piers and termite damage is evident. Any retrofit would require that all connections be properly upgraded, additional bracing be added and damaged lumber be removed or sistered with new lumber.

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SUMMARY - FINAL SLIDE | D-81





547 MEETING STREET SEPTEMBER 2, 2019

### ORIGINAL DEMOLITION SUBMITTAL



Requesting permission for full demolition of the structure at 547 Meeting Street. The front building was constructed between 1944 and 1951. Two rear additions were added at a later date. The existing structure does not exhibit any architectural merit nor are there any building materials worth salvaging.

COVER SHEET | D-0



3P

547 MEETING STREET JULY 24, 2019 The Montford Group.

## SHEET LIST

D-2 - AERIAL VIEWS D-3 - ARIAL VIEW D-4 - SANBORN MAP 1944 D-5 - SANBORN MAP 1951 D-11 - EXTERIOR PHOTOS D-12 - EXTERIOR PHOTOS D-13 - EXTERIOR PHOTOS D-14 - EXTERIOR PHOTOS D-15 - EXTERIOR PHOTOS D-16 - INTERIOR PHOTOS D-17 - INTERIOR PHOTOS D-18 - INTERIOR PHOTOS D-19 - INTERIOR PHOTOS D-20 - INTERIOR PHOTOS

SHEET LIST | D-1



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547 MEETING STREET JULY 24, 2019







BIRD'S EYE PHOTO 1

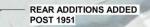


BIRD'S EYE PHOTO 2









ROOF IS FRAMED WITH ROOF TRUSSES THAT ARE COMPOSED OF 2X10 ENGINEERED LUMBER. ROOFING IS ASPHALT SHINGLE

EXTERIOR WALLS ARE BLOCK WITH STUCCO EXTERIOR. THERE ARE ALSO INTERMITTENT BRICK PILASTERS ON THE INTERIOR.

CONCRETE FLOOR -

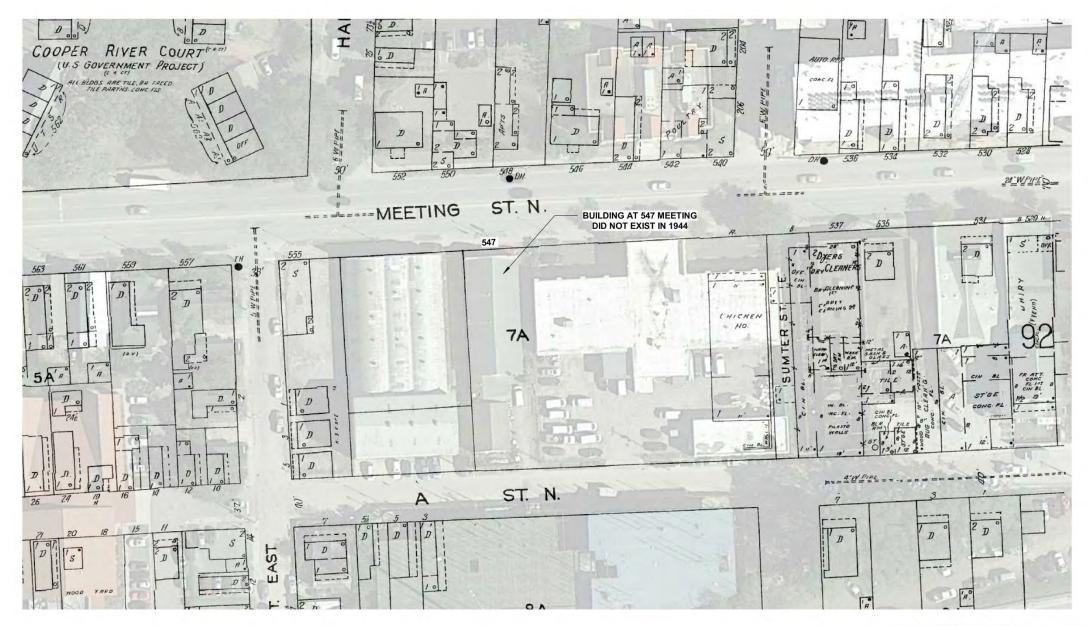
WOOD STUD PARAPET WALL WITH STUCCO EXTERIOR FRONT BUILDING AT 547 CONSTRUCTED BETWEEN 1944 AND 1951

## AERIAL VIEW | D-3





The Montford Group. We dare to dream, and create.



SANBORN MAP - 1944

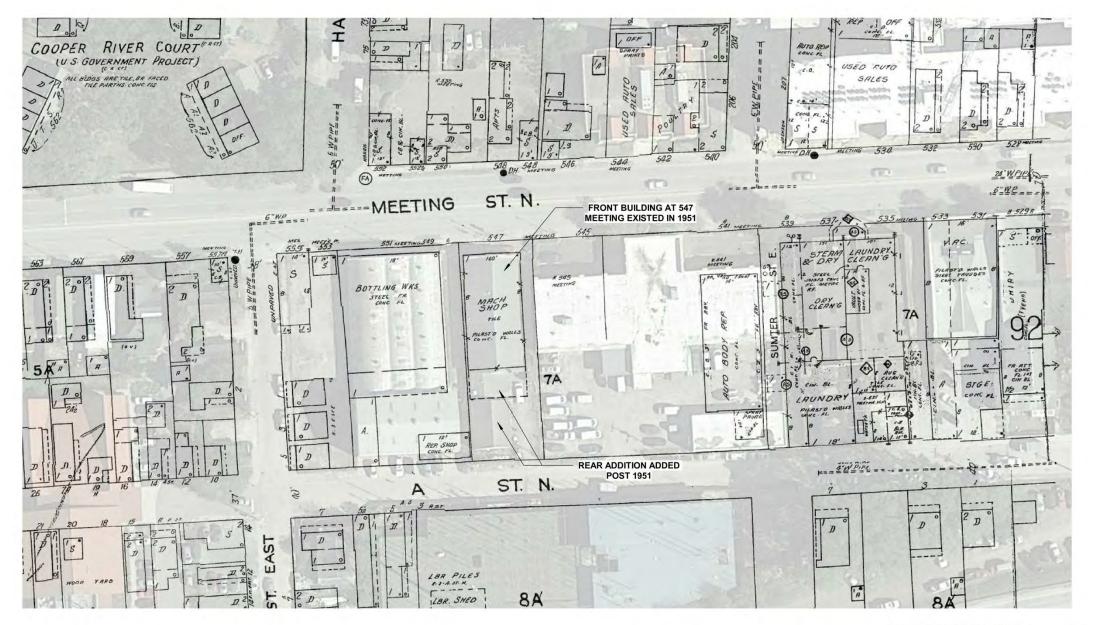
MEG

D-4

LS3P



547 MEETING STREET JULY 24, 2019



SANBORN MAP - 1951

D-5





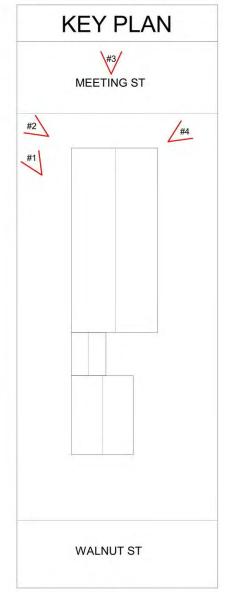


#4



#3





EXTERIOR PHOTOS | D-11





## 547 MEETING STREET JULY 24, 2019

-----

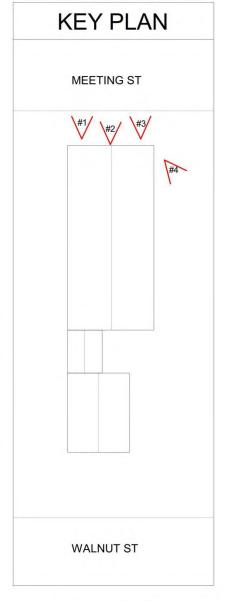




#4



#3



EXTERIOR PHOTOS | D-12



The Montford Group.





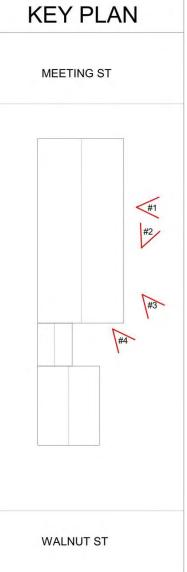
#2



#3



#4



EXTERIOR PHOTOS | D-13

LS3P









#2

#4



#3



**KEY PLAN** 

MEETING ST

#3 (#1 #2 WALNUT ST

> EXTERIOR PHOTOS | D-14



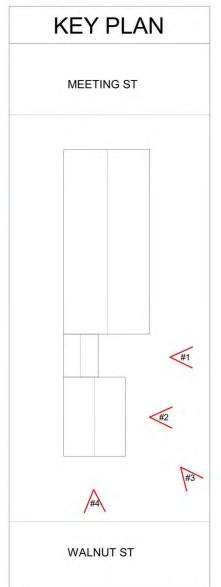












EXTERIOR PHOTOS | D-15

ARCHITECTURE

D-10

LSJP







#3

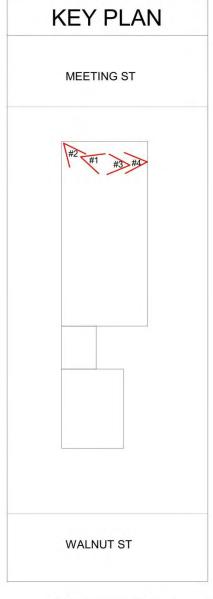








#4



INTERIOR PHOTOS | D-16

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547 MEETING STREET JULY 24, 2019

#3



INTERIOR PHOTOS | D-17

The Montford Group.



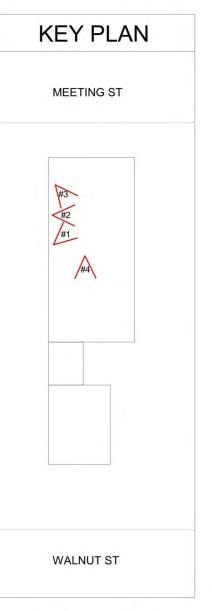








#4



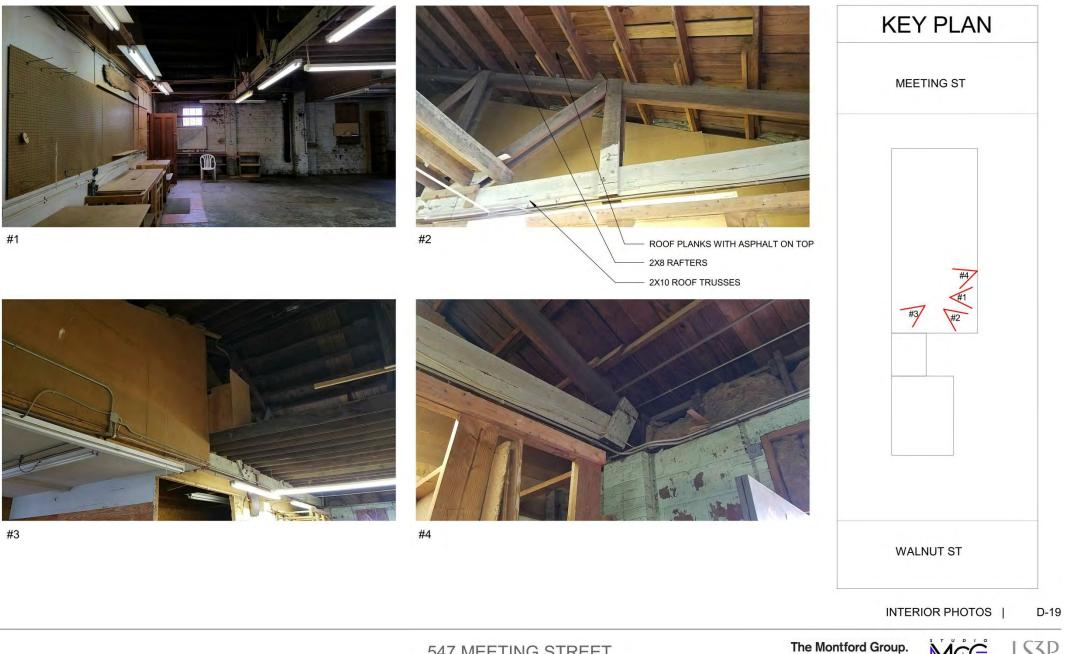
INTERIOR PHOTOS | D-18



The Montford Group.

We dare to dream, and create.

#3







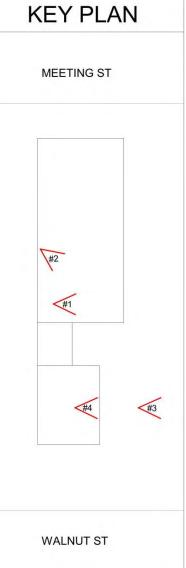




#3



#4



INTERIOR PHOTOS | D-20



The Montford Group.

Agenda Item 5:

# 810 Meeting Street - - TMS # 461-09-01-010

Request approval for demolition of existing structure.

Not Rated / (East Central) / c. 1951-55 / Historic Corridor District



## DEMOLITION REQUEST APPLICATION

A request is being made for the full demolition of the structure at 810 Meeting Street. A demolition request was granted unanimous approval at the BAR-S on July 27, 2017. Since that approval, there have been no changes to the building that would deem it a structure worth preserving. The building was built between 1951 and 1955. It is a non-contributing building that is detrimental to the streetscape. Furthermore the building is functionally obsolete for anything other than a convenience store..

### SHEET LIST

- D1 COVER SHEET
- D2
- D3 D4
- D5
- D6
- BIRD'S EYE VIEW SANBORN ANALYSIS EXTERIOR PHOTOS EXTERIOR PHOTOS EXTERIOR PHOTOS INTERIOR PHOTOS D7
- D8 INTERIOR PHOTOS

COVER SHEET



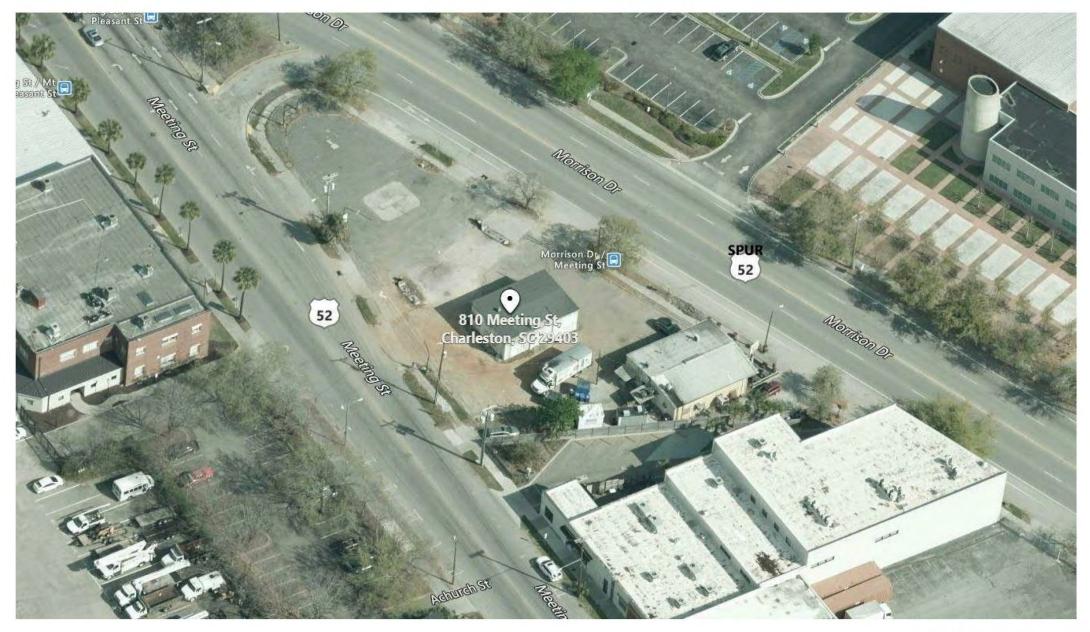
The Montford Group.

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D1

810 MEETING STREET DEMO REQUEST AUGUST 28, 2019



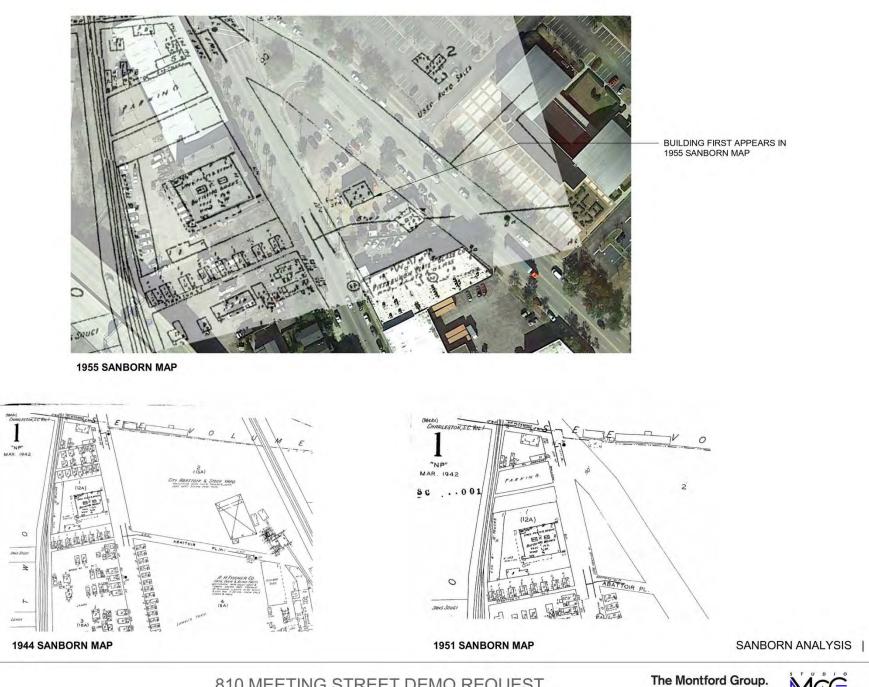
810 MEETING STREET DEMO REQUEST AUGUST 28, 2019

# BIRD'S EYE VIEW |

## D2









D3

810 MEETING STREET DEMO REQUEST AUGUST 28, 2019



2 PHOTO 2

810 MEETING STREET DEMO REQUEST

AUGUST 28, 2019



KEY PLAN

EXTERIOR PHOTOS | D4

LS3P







4 PHOTO 4



KEY PLAN

EXTERIOR PHOTOS |

LS3P MEG

D5





AUGUST 28, 2019



6 PHOTO 6

810 MEETING STREET DEMO REQUEST

AUGUST 28, 2019



KEY PLAN

EXTERIOR PHOTOS | D6



The Montford Group.







8 PHOTO 8

810 MEETING STREET DEMO REQUEST

AUGUST 28, 2019

INTERIOR PHOTOS |



LSJP

D7

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AUGUST 28, 2019

9 PHOTO 9

10 PHOTO 10



# D8







Agenda Item 6:

529 King Street - - TMS # 460-12-02-081

Request preliminary approval for new construction of mixed-used building to include retail and hotel.

(Cannon-Elliottborough) / Old and Historic District

# 529 King Street, Charleston Preliminary BAR Drawings

Submission Date: September 03, 2019 Meeting Date: September 11, 2019

529 King Street, Charleston, SC 29403

Roost Charleston Method Residential

529 King Street

Charleston, SC 29403

METHOD

Marris Adjmi Architects

www.ma.com

MA

Previous Approvals Description:

March 27, 2019 - CONCEPTUAL BAR APPROVAL

At the March 27th Conceptual BAR Meeting the project received high praise from the Board members including the comment that the design did not need to be "messed with". The project received unanimous approval including an additional half story at the 5th floor.

## DRAWING LIST - 529 KING STREET:

SECTION 1 - BAR PRESENTATION DRAWINGS

SECTION 2 - ARCHITECTURAL WORKING DRAWINGS

Construction Subsystem

Site Plan

First Floor Plan Second Floor Plan

Third Floor Plan

Fifth Floor Plan Roof Floor Plan

Fourth Floor Plan

Bulkhead Floor Plan Exterior Elevations

Exterior Elevations Building Sections

**Building sections** 

Plan Details

Section Details (Front Facade)

Section Details (Penthouse)

Section Details (West Facade)

Plan and Section Details (Storefront)

King Street Enlarged Elevation and Wall Sections

StoreFront Enlarged Elevation and Wall Sections

Penthouse Enlarged Elevation and Wall Sections

Rear Facade Enalrged Elevation and Wall Sections

A-001

A-002

A-101

A-102 A-103

A-104

A-105

A-106 A-107

A-201 A-202

A-301

A-302

A-310

A-311

A-312 A-313

A-320

A-321 A-322

A-323

A-324

| BAR-000 | Drawing List                                  |  |
|---------|---|--|
| BAR-001 | King Street Facade/BAR Summary                |  |
| BAR-002 | King Street Facade/BAR Summary 2              |  |
| BAR-003 | King Street Facade/BAR Summary 3              |  |
| BAR-004 | Site Photos                                   |  |
| BAR-101 | Ground Floor Plan Comparison                  |  |
| BAR-102 | Second Floor Plan Comparison                  |  |
| BAR-103 | Third Floor Plan Comparison                   |  |
| BAR-104 | Fourth Floor Plan Comparison                  |  |
| BAR-105 | Fifth Floor Plan Comparison                   |  |
| BAR-106 | Roof/Bulkhead Floor Plan Comparison           |  |
| BAR-201 | East Elevation Comparison                     |  |
| BAR-202 | West Elevation Comparison                     |  |
| BAR-203 | North Elevation Comparison                    |  |
| BAR-204 | South Elevation Comparison                    |  |
| BAR-211 | Streetscape with 529 King Facade              |  |
| BAR-212 | Building Street Views                         |  |
| BAR-221 | King Street Facade Rendering                  |  |
| BAR-222 | Storefront Rendering                          |  |
| BAR-223 | Rear Facade Rendering                         |  |
| BAR-224 | Penthouse Rendering                           |  |
| BAR-225 | Courtyard Study & Sketches                    |  |
| BAR-226 | Courtyard Rendering                           |  |
| BAR-401 | King Street Elevation Perspective and Details |  |
| BAR-402 | Precast Concrete Panel Layout Diagram         |  |
| DAD (00 | Dhusiaal Madal Dhataa                         |  |

BAR-403 Physical Model Photos

**Drawing List** 

Freliminary BAR Review / 03 September 2019

529 King Street

Charleston, SC 29403

METHOD

Marris Adjmi Architects

www.ma.com

MA

- BAR-404 Rear Elevation Perspective and Details
- BAR-601 Proposed Material Palette





Key Board Comments from Conceptual BAR

LS3P

Morris Adjmi Architects

www.ma.com

MA

Conceptual BAR Approval with staff comments 2 and 3 and Board comment that the architectural merit warrants allowing additional half story  $% \mathcal{A} = \mathcal{A}$ 

Staff Comment 2: The parking entrance in the rear is a welcomed move

METHOD

Staff Comments 3: There is an unusually large area of mechanical on the roof which is very prominent from St. Phillips Street

529 King Street Charleston, SC 29403 King Street Facade - BAR Summary

Preliminary BAR Review / 03 September 2019

### CURRENT DESIGN - PRELIMINARY BAR

#### Key Refinements since Conceptual BAR

1. Storefront design has been refined and developed with additional wood trim detailing

2. Canopy design and its curve has been refined for constructability. Wood ceiling at canopy has been proposed to recall local porch architecture

3.Window size has been further developed

4. Precast panel side returns have been widened and have added scoring details

5. The joints of the precast facade have been refined for constructability and aesthetics



BAR-001



Key Board Comments from Conceptual BAR

Conceptual BAR Approval with staff comments 2 and 3 and Board comment that the architectural merit warrants allowing additional half story

Staff Comment 2: The parking entrance in the rear is a welcomed move

Staff Comments 3: There is an unusually large area of mechanical on the roof which is very prominent from St. Phillips Street



#### CURRENT DESIGN - PRELIMINARY BAR

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4. Precast panel side returns have been widened and have added scoring details

5. The joints of the precast facade have been refined for constructability and aesthetics

6. Penthouse height has increased 12"



MA

Facade Street View (Looking South) - BAR Summary Preliminary BAR Review / 03 September 2019





#### Key Board Comments from Conceptual BAR

LSSP

Morris Adjmi Architects www.ma.com

MA

Conceptual BAR Approval with staff comments 2 and 3 and Board comment that the architectural merit warrants allowing additional half story

METHOD

Staff Comment 2: The parking entrance in the rear is a welcomed move

Staff Comments 3: There is an unusually large area of mechanical on the roof which is very prominent from St. Phillips Street

529 King Street Charleston, SC 29403

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3.Window size has been further developed

4. Precast panel side returns have been widened and have added scoring details

5. The joints of the precast facade have been refined for constructability and aesthetics

6. Penthouse height has increased 12"

7. Wood Burning Fireplace has been added to the Penthouse



Facade Street View (Looking North) - BAR Summary Preliminary BAR Review / 03 September 2019





РНОТО 3







King Street



529 King Street Charleston, SC 29403





PHOTO 5

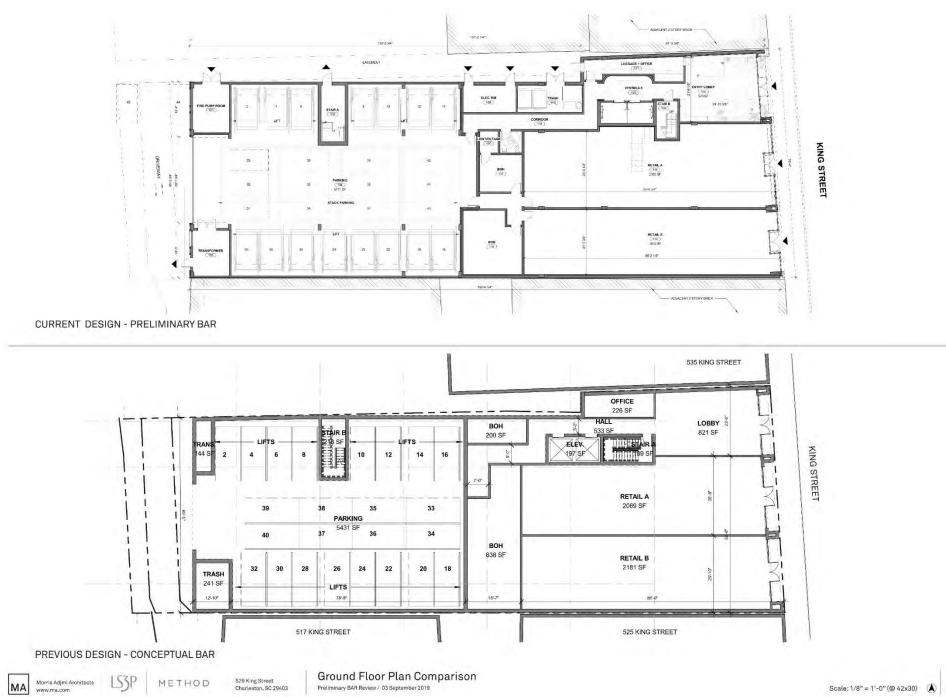


Driveway/St. Philip

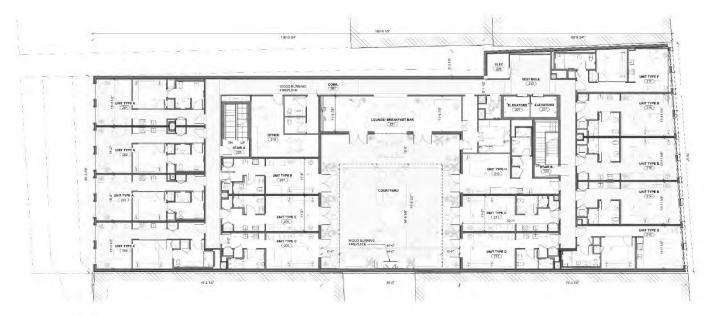
MA Morris Adjmi Architects LSSP METHOD

Site Photos: King Street and Driveway off St. Philips Preliminary BAR Review / 03 September 2019

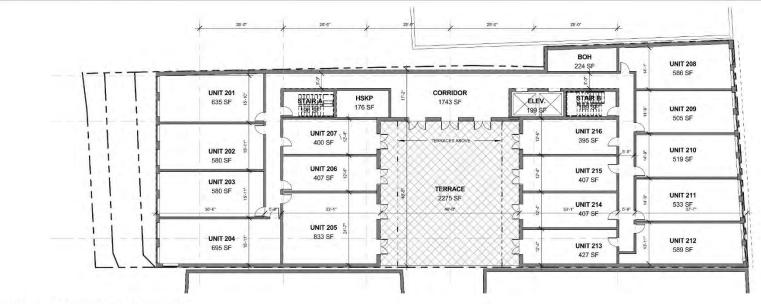
BAR-004



Scale: 1/8" = 1'-0" (@ 42x30) 🚯 BAR-101

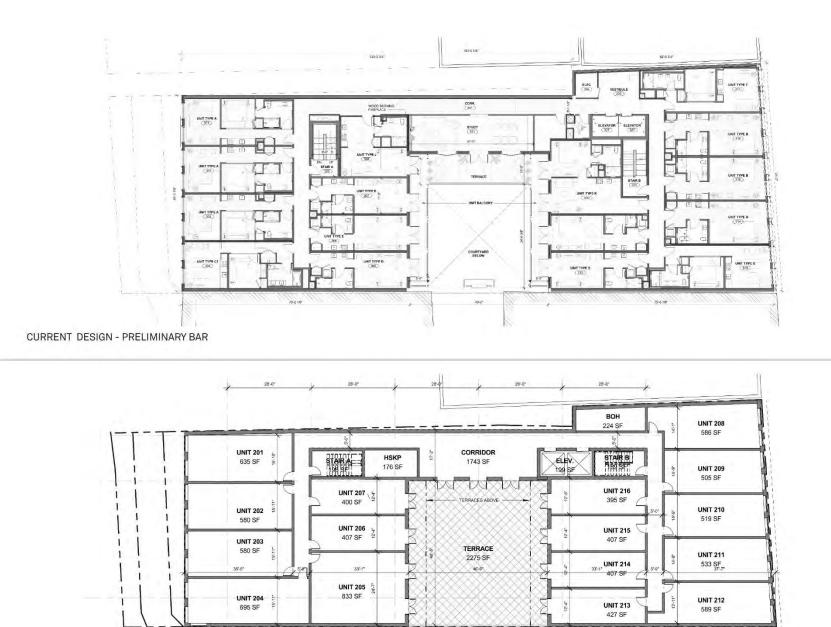


CURRENT DESIGN - PRELIMINARY BAR



PREVIOUS DESIGN - CONCEPTUAL BAR

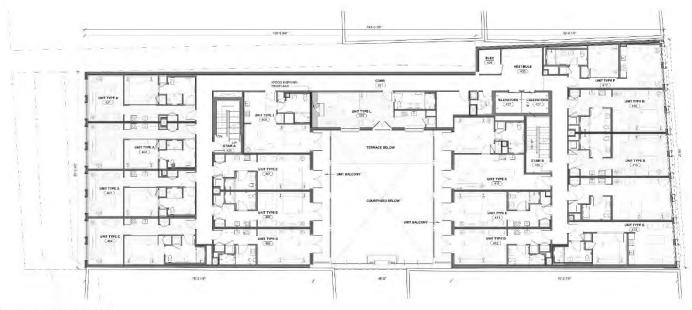




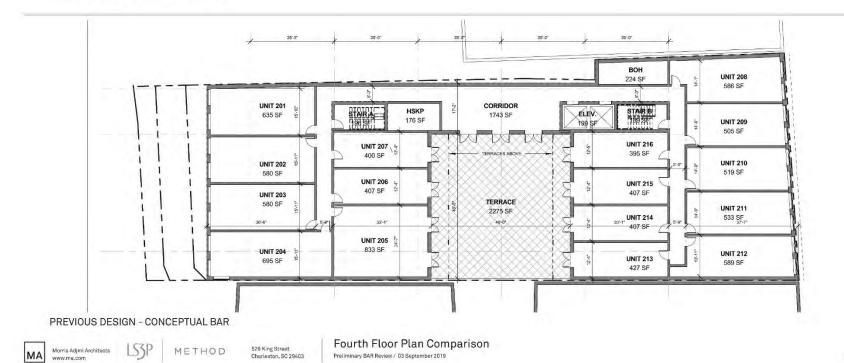
MA Morris Adjimi Architects LSSP METHOD 529 King Street Charleston, SC 29403

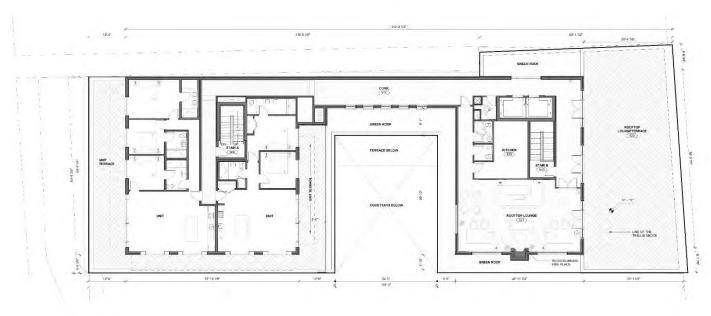
Third Floor Plan Comparison Preliminary BAR Review / 03 September 2019

Scale: 1/8" = 1'-0" (@ 42x30) A BAR-103

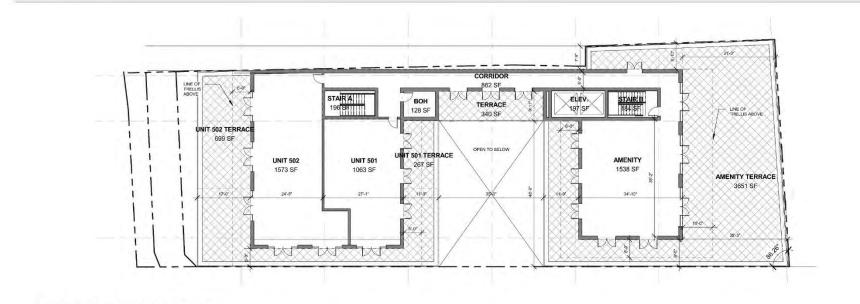


CURRENT DESIGN - PRELIMINARY BAR



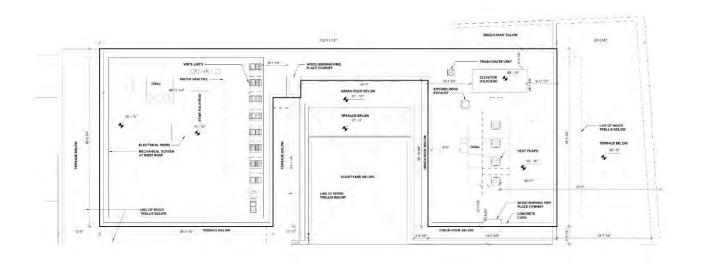


CURRENT DESIGN - PRELIMINARY BAR

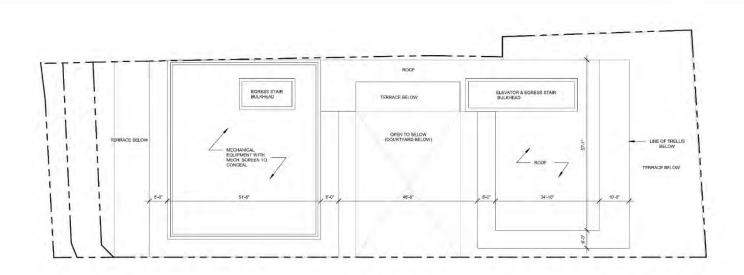


PREVIOUS DESIGN - CONCEPTUAL BAR





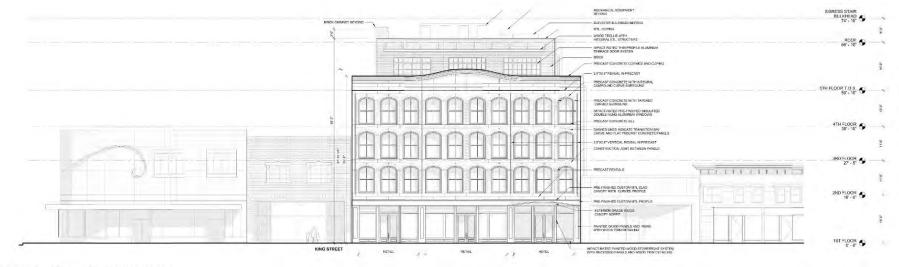
CURRENT DESIGN - PRELIMINARY BAR



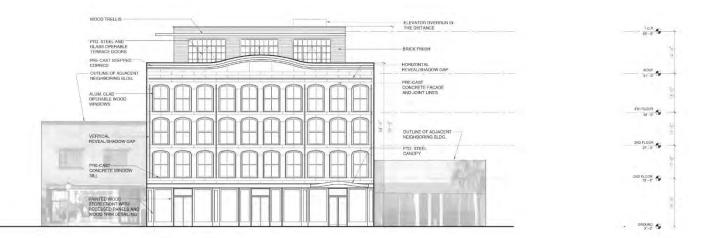
PREVIOUS DESIGN - CONCEPTUAL BAR



Roof/Bulkhead Floor Plan Comparison Freliminary BAR Review / 03 September 2019

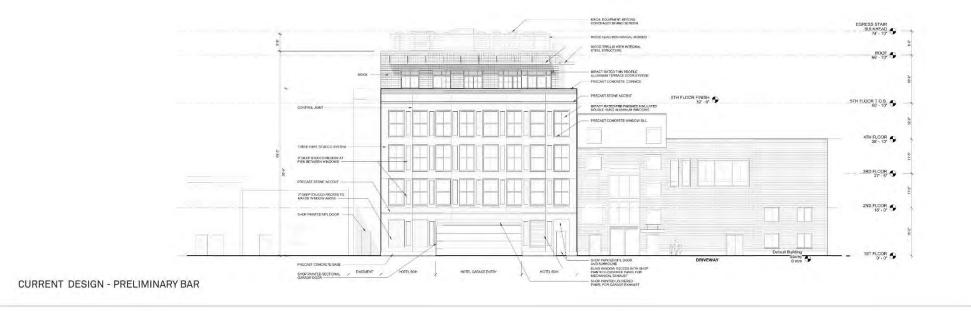


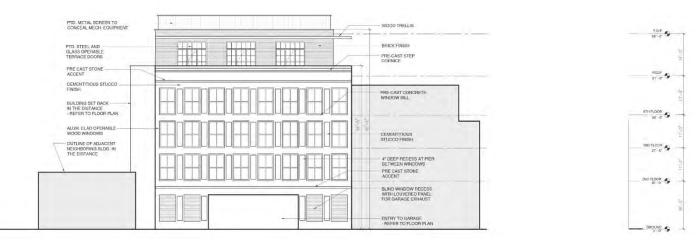
CURRENT DESIGN - PRELIMINARY BAR



PREVIOUS DESIGN - CONCEPTUAL BAR



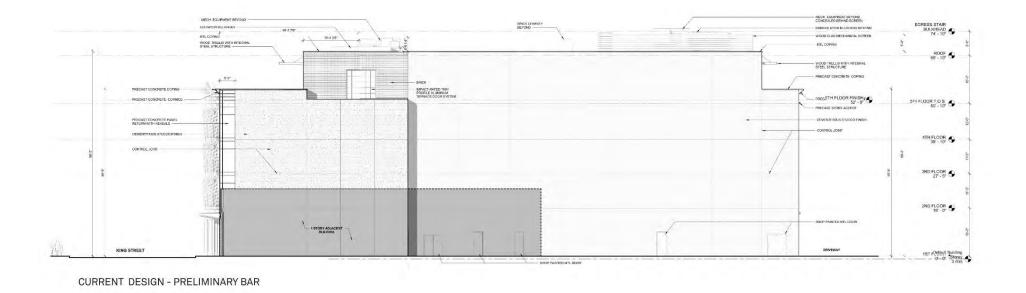




# PREVIOUS DESIGN - CONCEPTUAL BAR



### RearWest Elevation Comparison Preliminary BAR Review / 03 September 2019



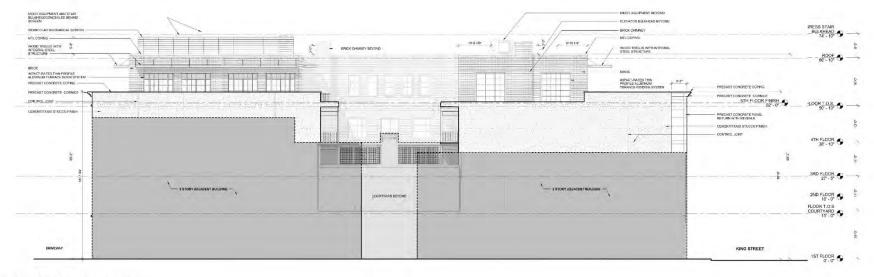
ELEVATOR OVERUN BULKEAD PTD. METAL SCREEN TO CONCEAL MECH. EQUIPMENT IT WOOD TRELLIS WOOD TRELLIS - T.O.P. Ĥ PRE-CAST COPING STONE OUTLINE OF THE PENTHOUSE SE CORNICE PEDIMENT IN THE DISTANCE ROOF PROFILE OF PRECAST CONCRETE FACADE AND SIDE RETURN - REFER TO KING ST. FACADE CEMENTITIOUS STUCCO SCORED CEMENTITIOUS -STUCCO ATH FLOOR LINE OF BUILDING SET BACK AT THE PROPERTY LINE SCORED CEMNETITIOUS STUCCO FINISH PROFILE OF EXISTING NEIGHBORING BUILDING 3 3RD FLOOR 2F - 0" PTD. STEEL-CANOPY 2ND FLOOR -KING STREET DRIVEWAY GROUND

PREVIOUS DESIGN - CONCEPTUAL BAR

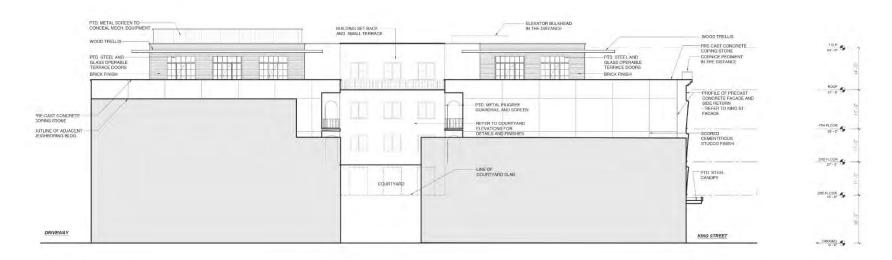


North Elevation Comparison Preliminary BAR Review / 03 September 2019

Scale: 1/8" = 1'-0" (@ 42x30) BAR-203



CURRENT DESIGN - PRELIMINARY BAR

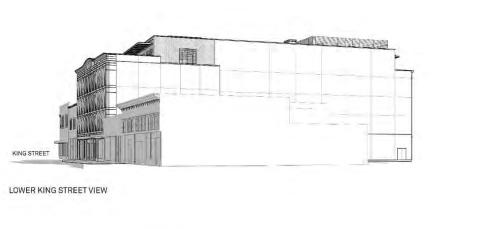


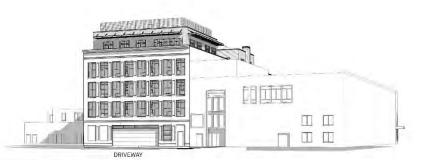
PREVIOUS DESIGN - CONCEPTUAL BAR





Enlarged Proposed King Street Streetscape





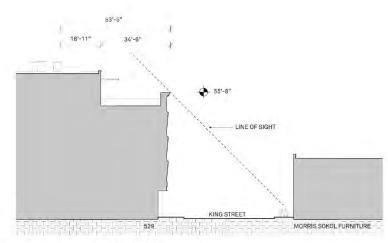




UPPER KING STREET VIEW



Street Views of the Overall Building Preliminary BAR Review / 03 September 2019



KING STREET SECTION WITH SIGHT LINE

MORRIS STREET

KEY PLAN / CAMERA LOCATION

ST PHILLIP & MORRIS

Scale: 1/8" = 1'-0" (@ 42x30) A BAR-212

UPPER KING













Palazzo dei Normanni, Palermo



Palazzo Strozzi, Florence



Dock Street Theater, Charleston, SC



Palazzo Medici Ricciardi, Florence

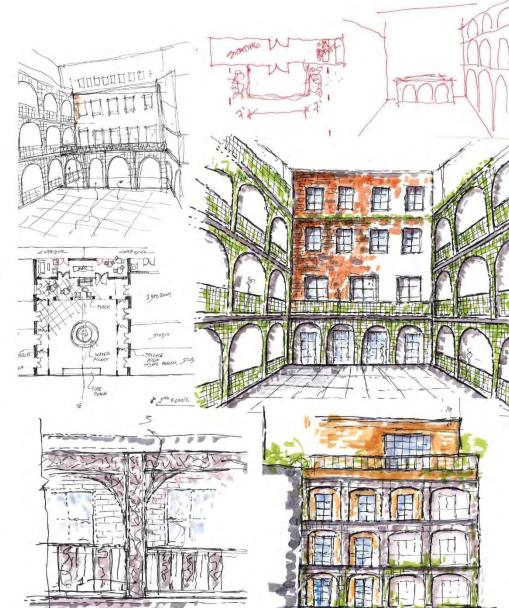
MA Morris Adjmi Architects www.ma.com

Renaissance Palace Precedents

LS3P

Charleston Precedents

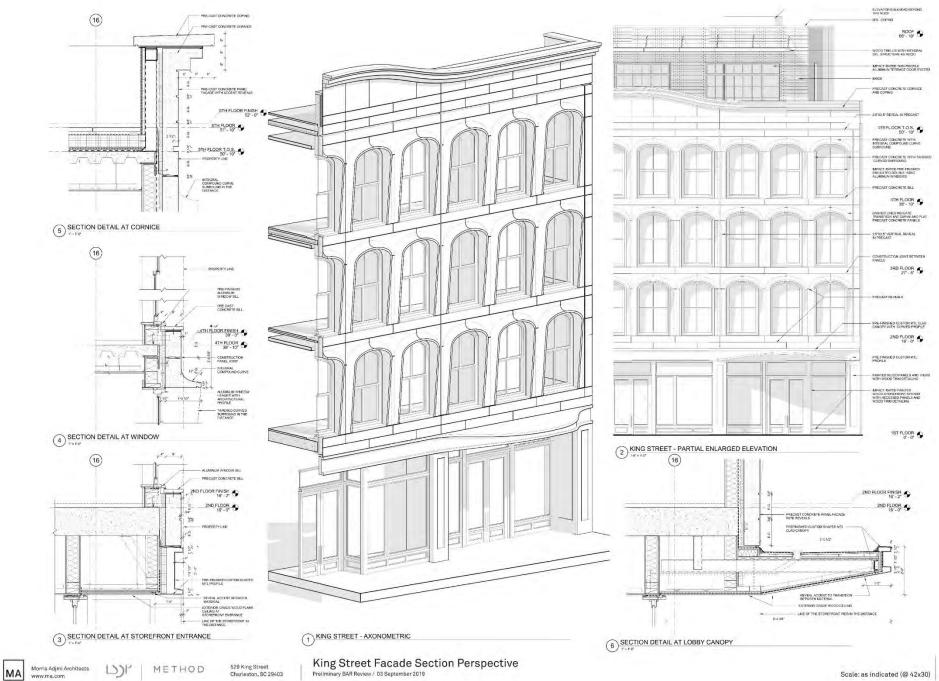
529 King Street Charleston, SC 29403



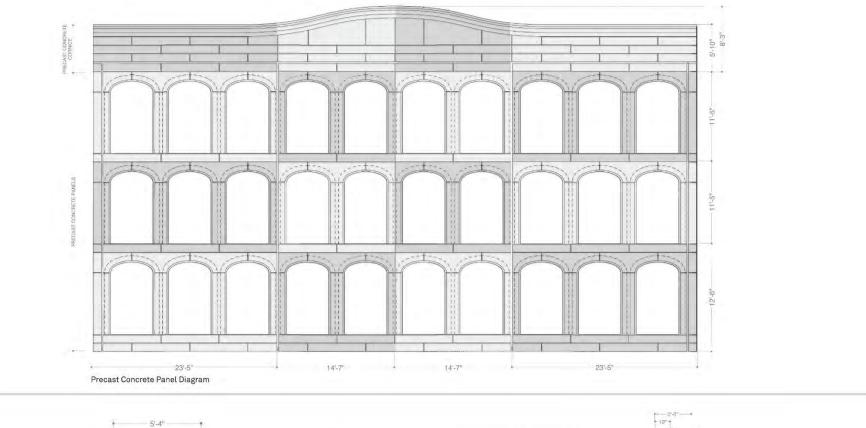


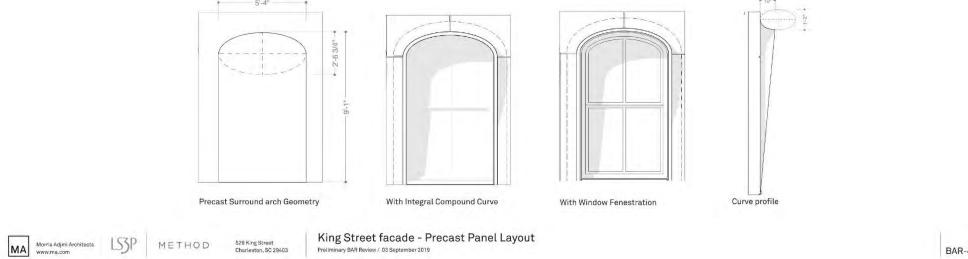
METHOD

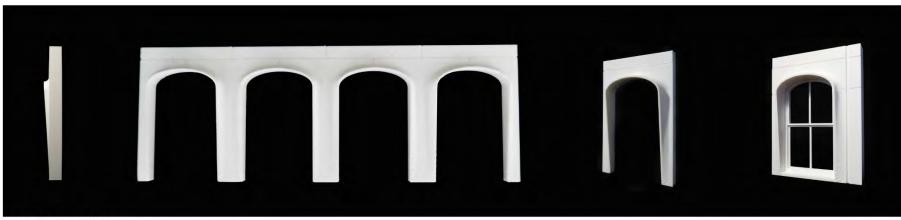




Scale: as indicated (@ 42x30) BAR-401







Details of the Compound Curve



Partial Model of the Facade

LS3P

METHOD

MA Morris Adjmi Architects www.ma.com



Partial Model of the Facade with Storefront and Canopy

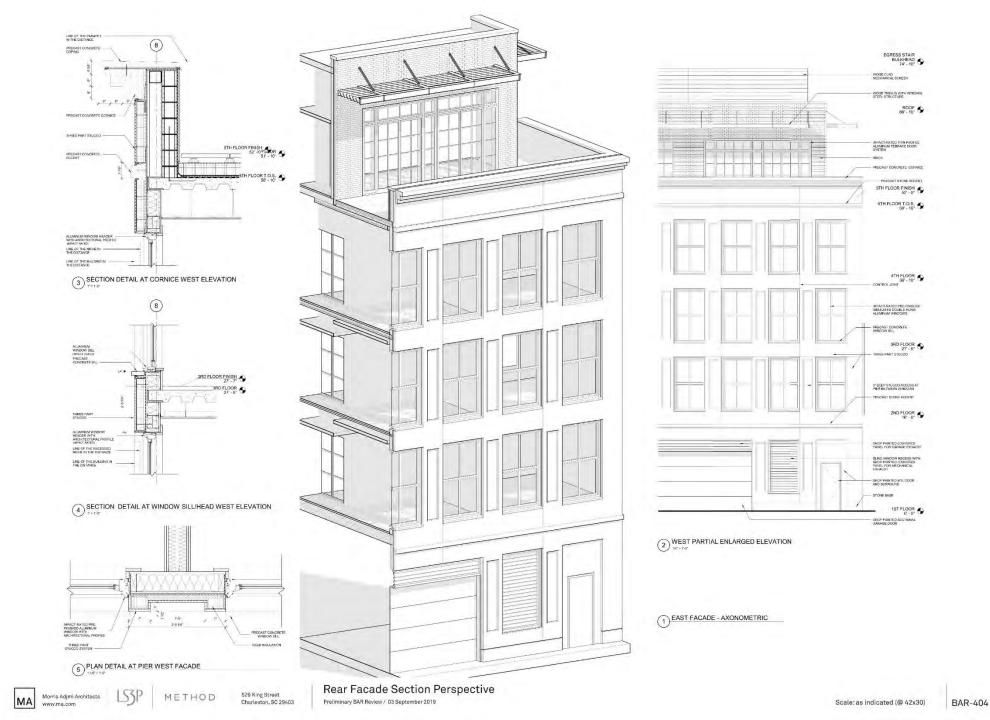




Detail of the Hotel Entry and Canopy



529 King Street Charleston, SC 29403 Design Study: Physical Model Photos Preliminary BAR Review / 03 September 2019





KING STREET ELEVATION (LOOKING SOUTH)

LS3P

METHOD

529 King Street Charleston, SC 29403

MA Morris Adjmi Architects www.ma.com

# MATERIAL #6 Three part Stucco



MATERIAL #5 Precast Concrete



MATERIAL #3/#4 Pre-finished Metal Window Frame and architectural fascia/Canopy clad





MATERIAL #1 Painted Wood Finish for Storefront Proposed Material Palette Summary

Preliminary BAR Review / 03 September 2019



PENTHOUSE



MATERIAL #9 Exterior Grade Natural Wood Trellis



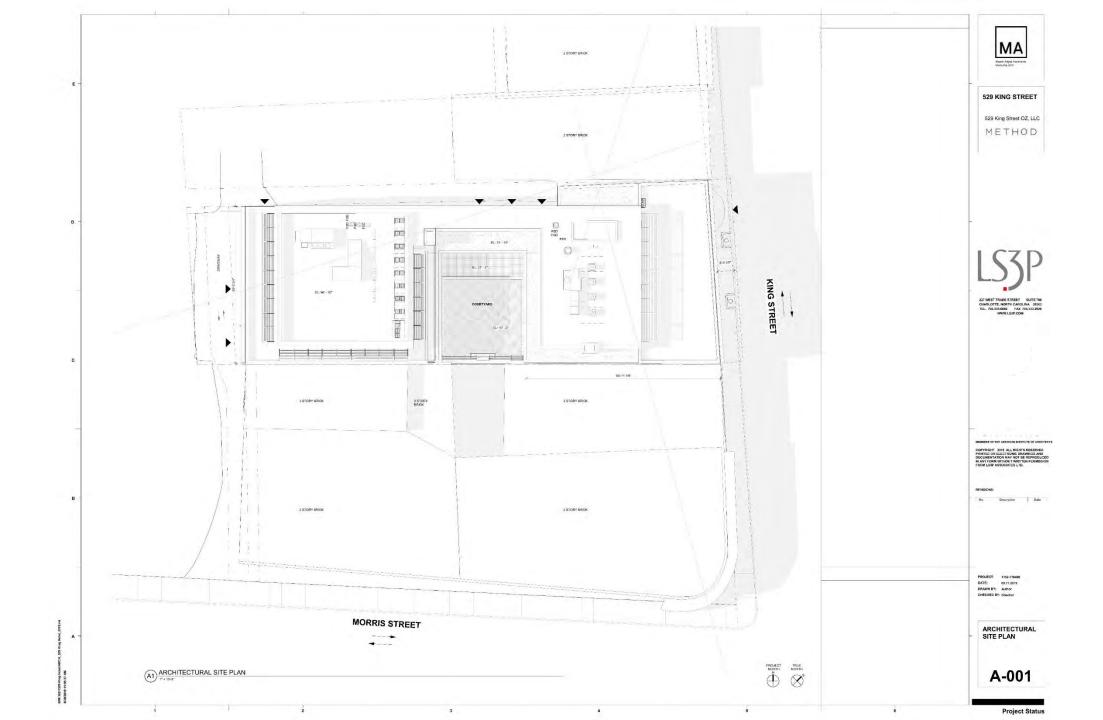
MATERIAL #8 White Washed Red Brick

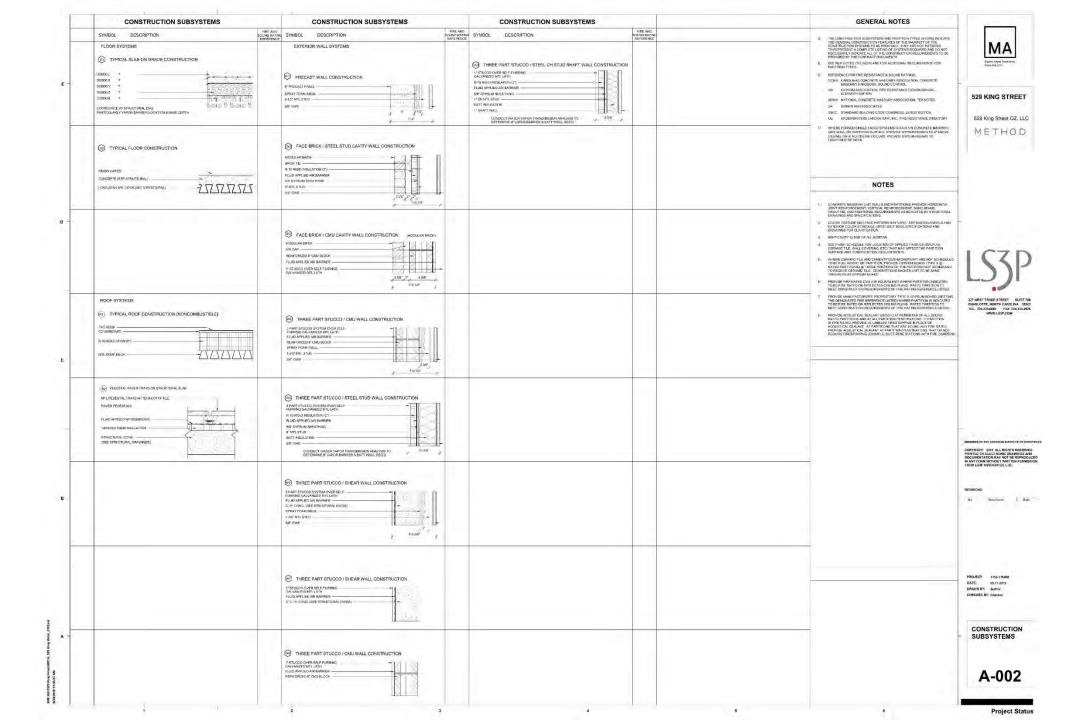


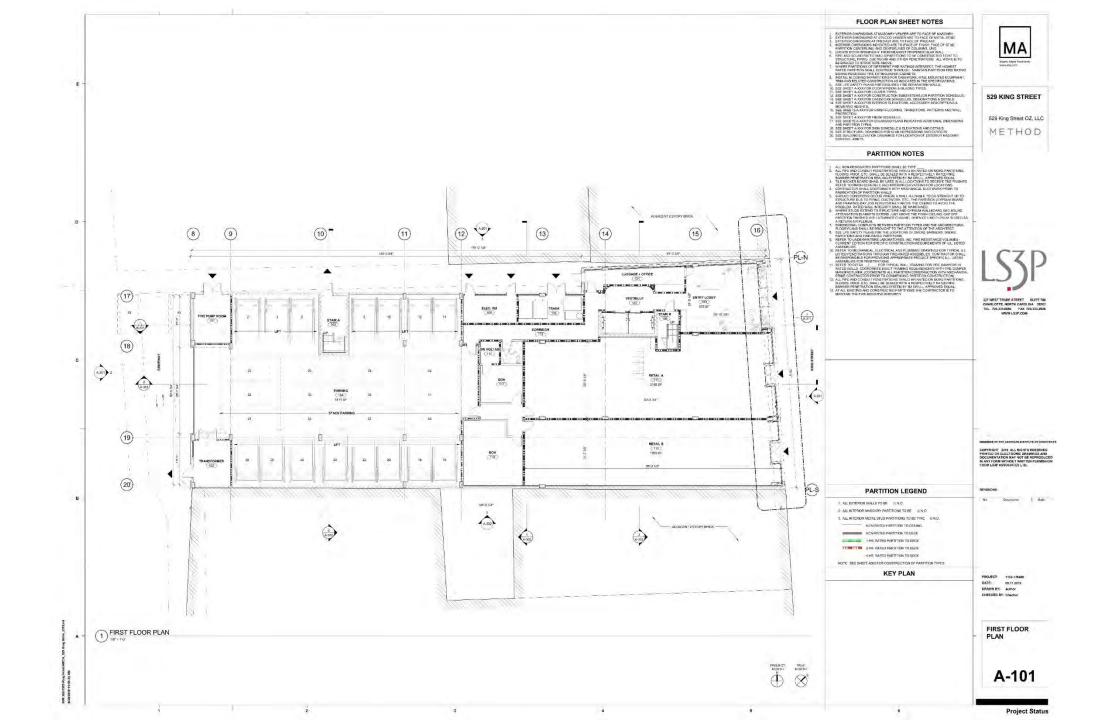
MATERIAL #7 Pre-Finished Metal for Terrace Doors

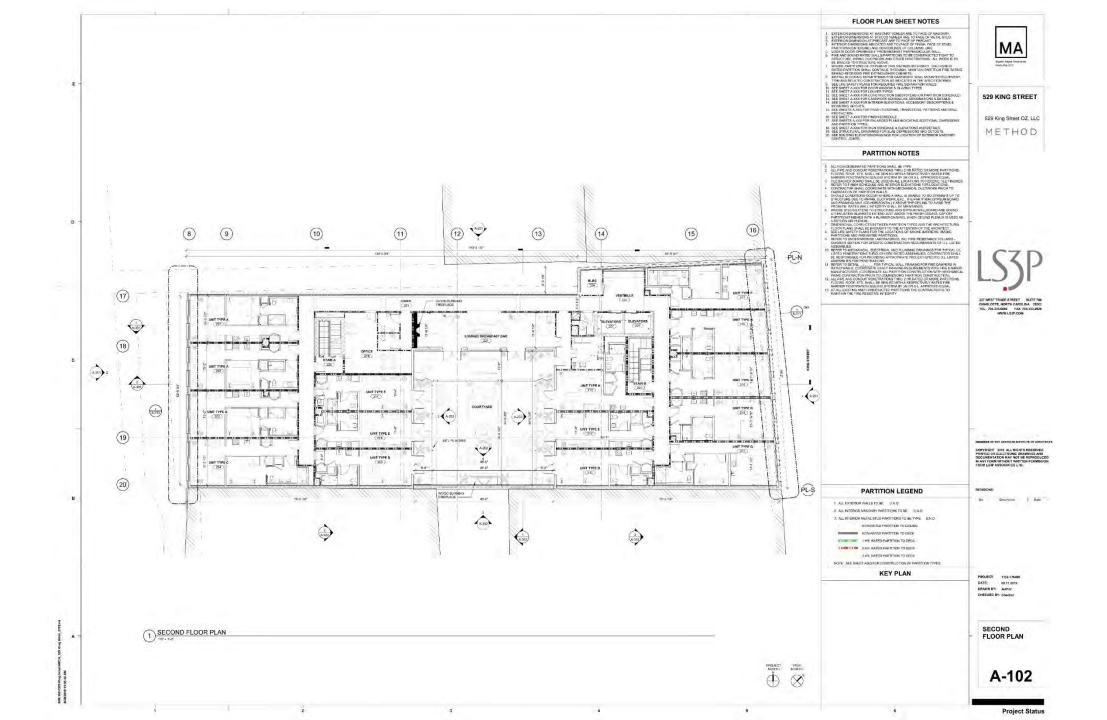
# WORKING DRAWINGS

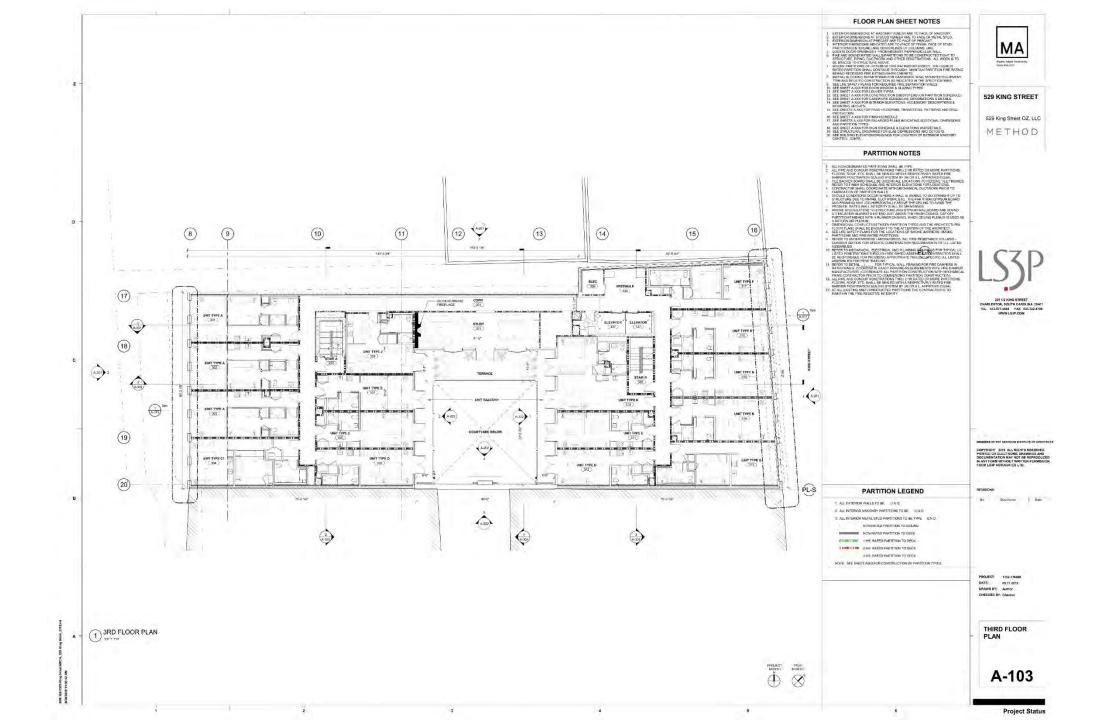
MA Marris Adjimi Architects LSJP METHOD 528 King Street www.ma.com S2 9403 Preliminary BAR Review/ 03 September 2019

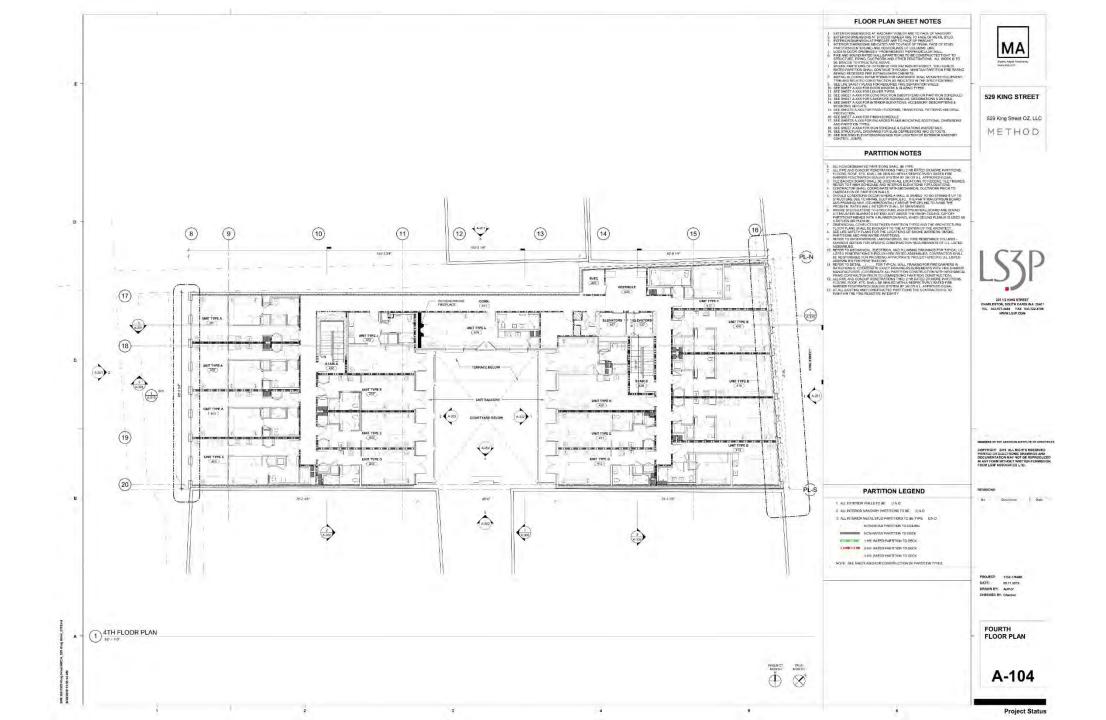


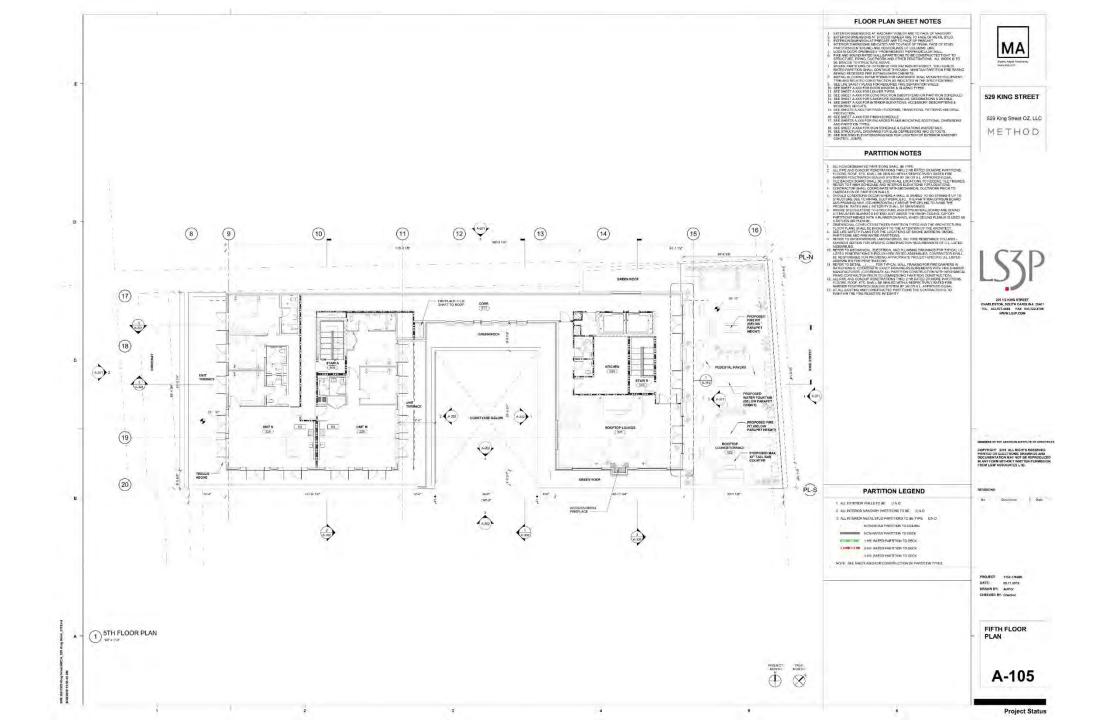


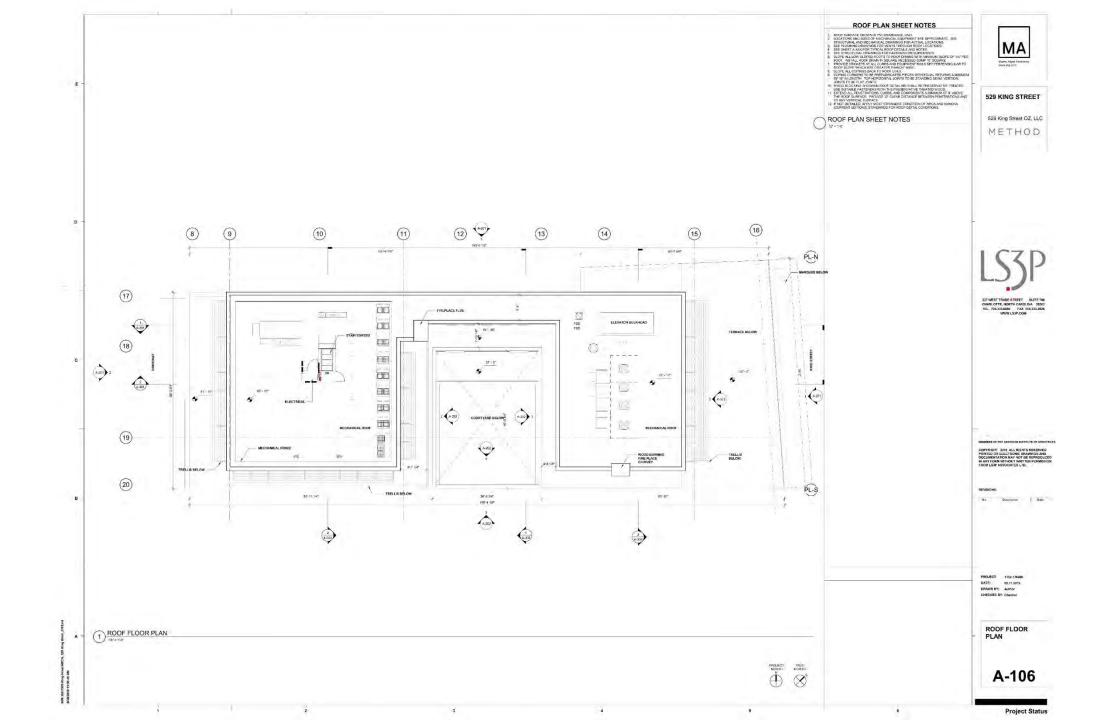


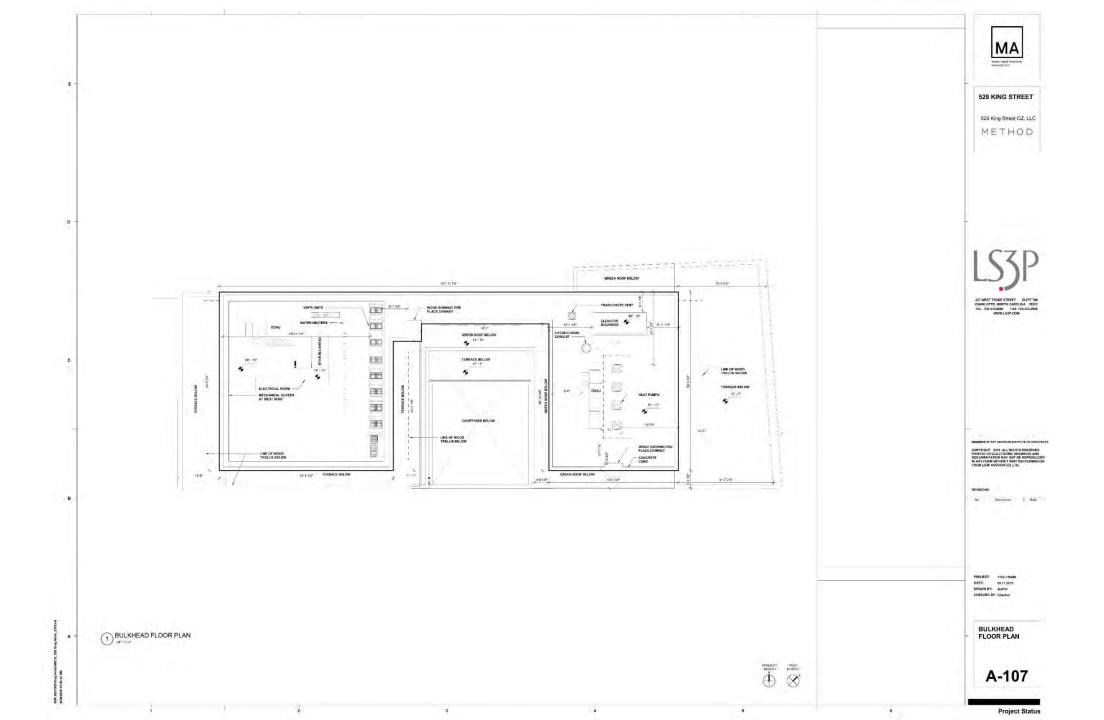


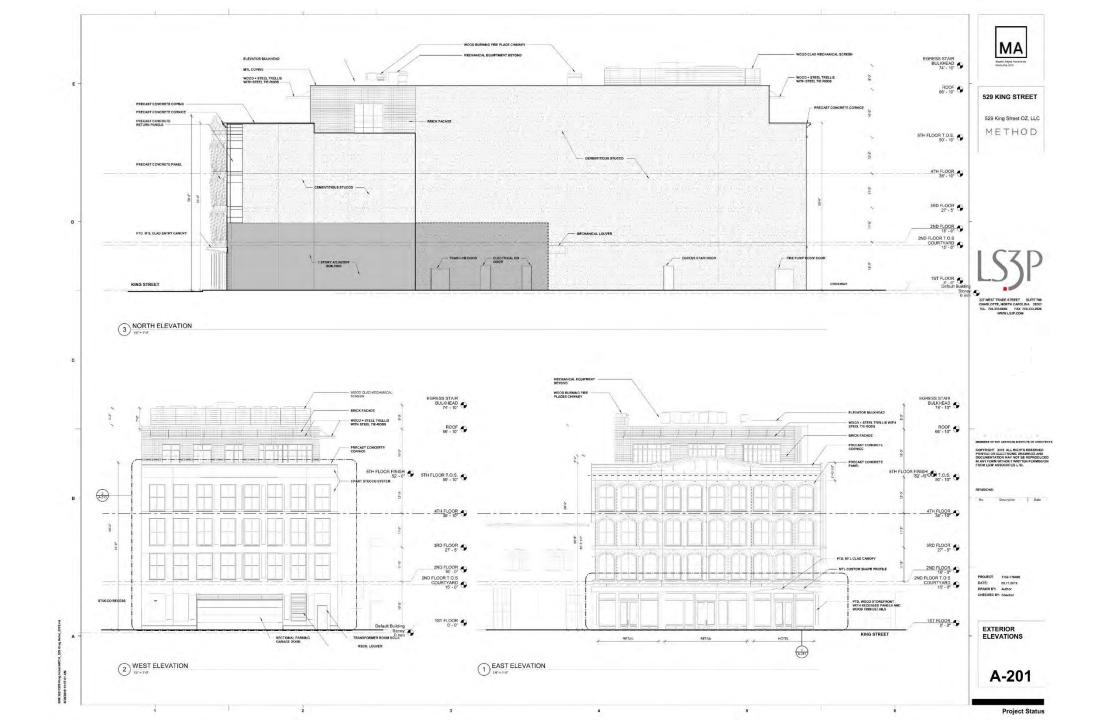


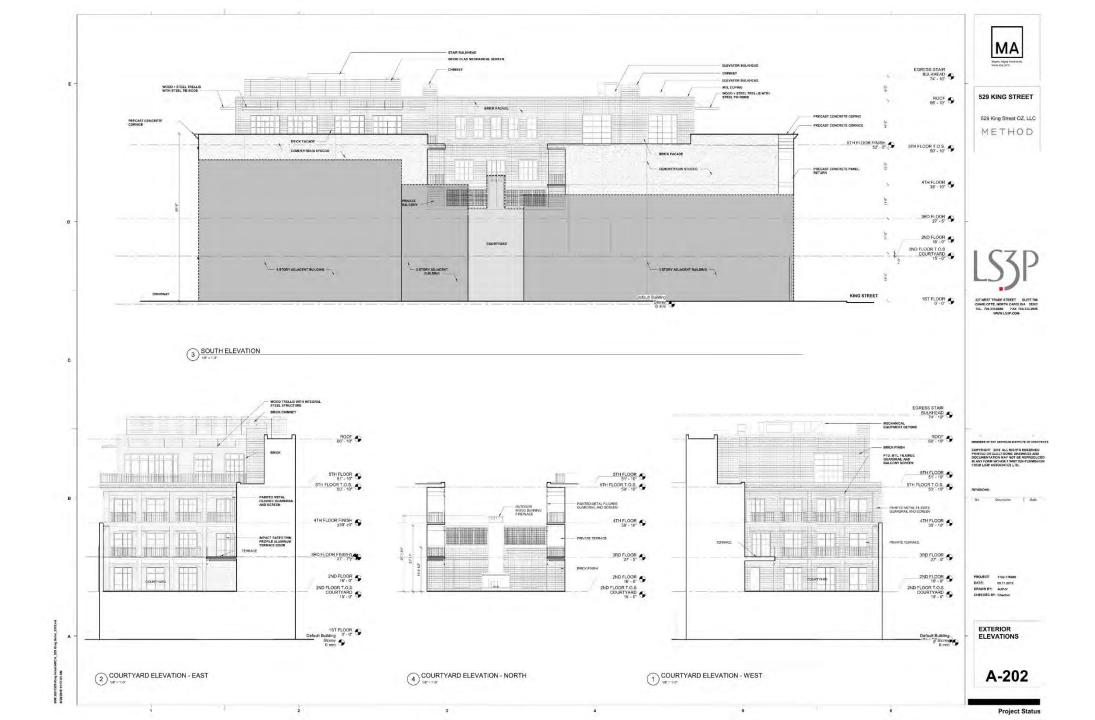


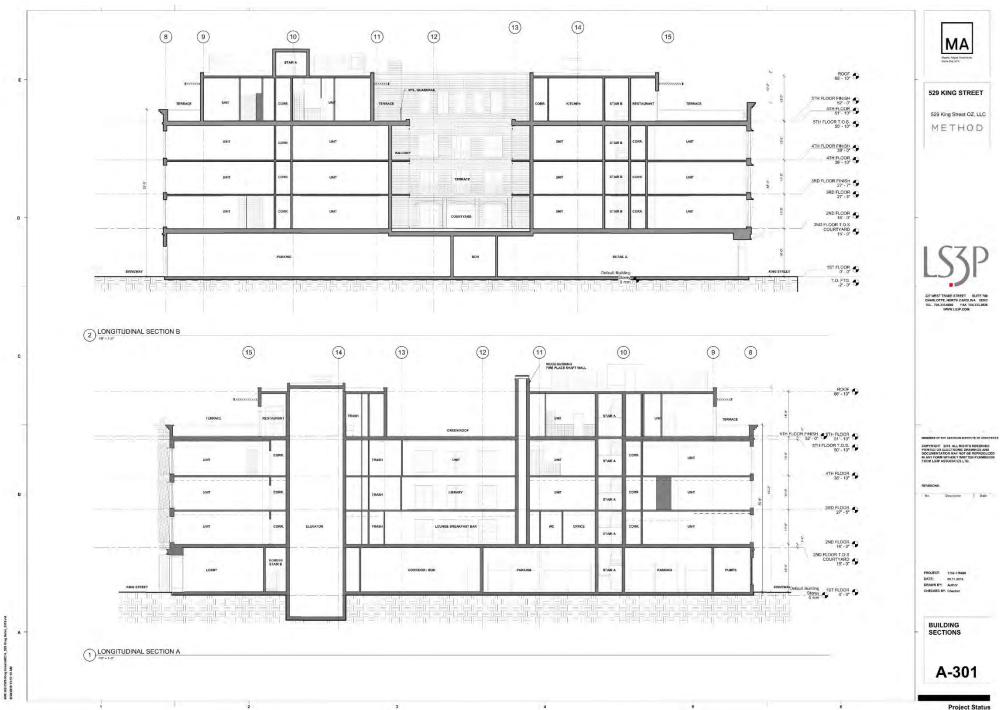




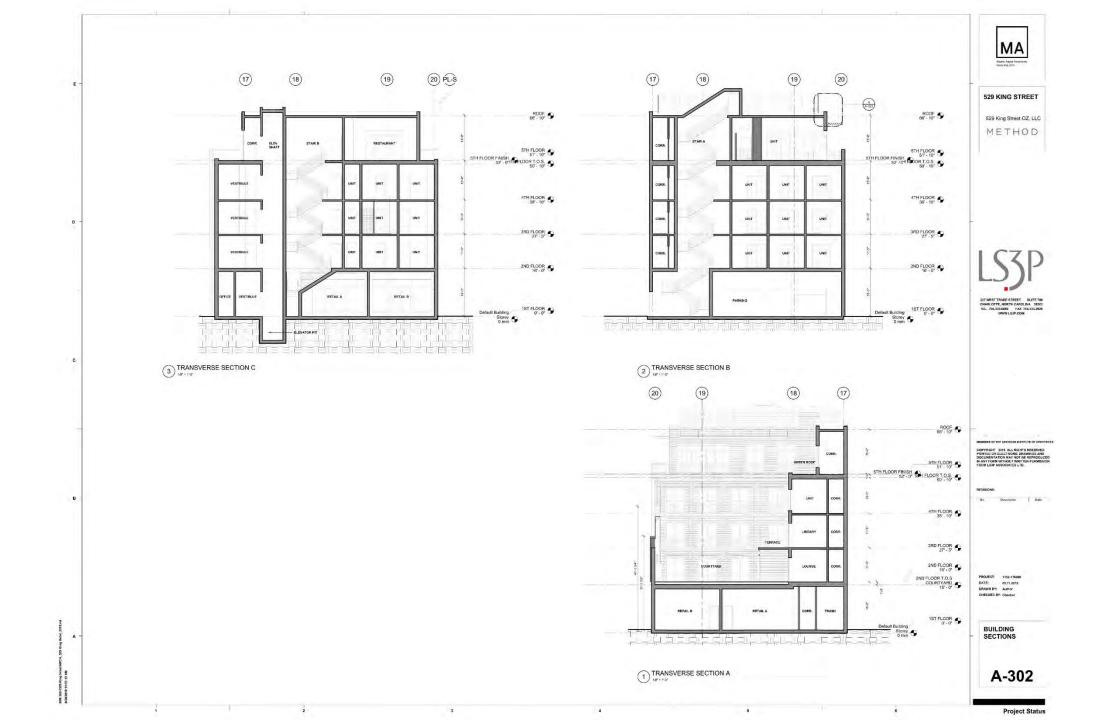


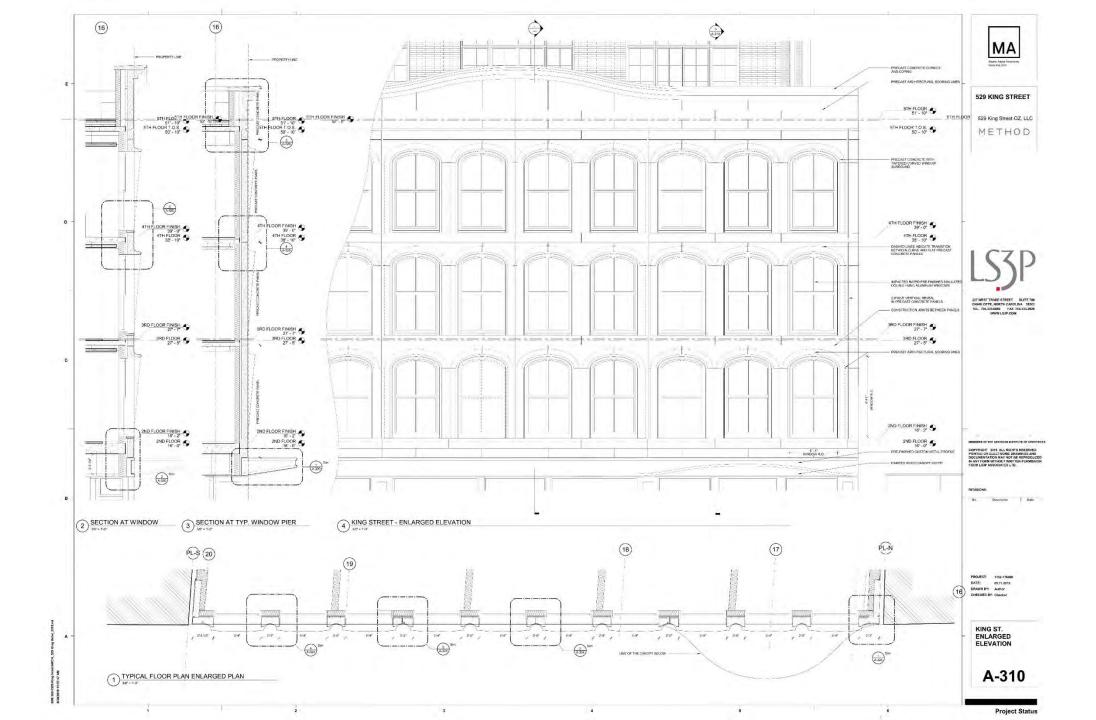


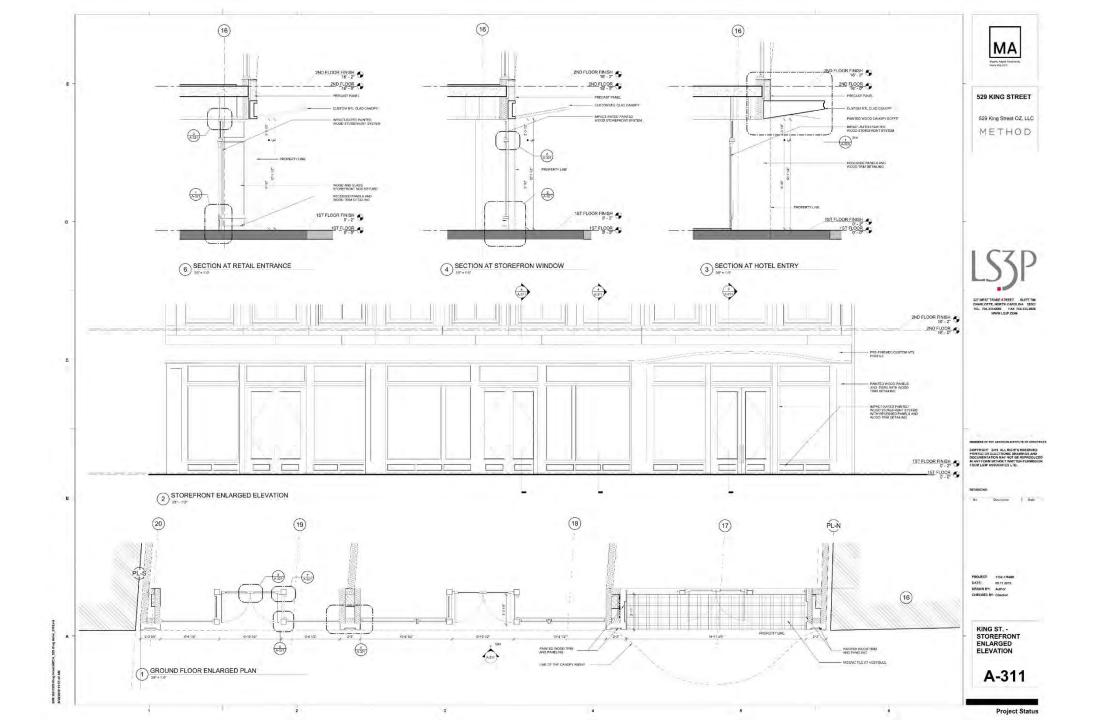


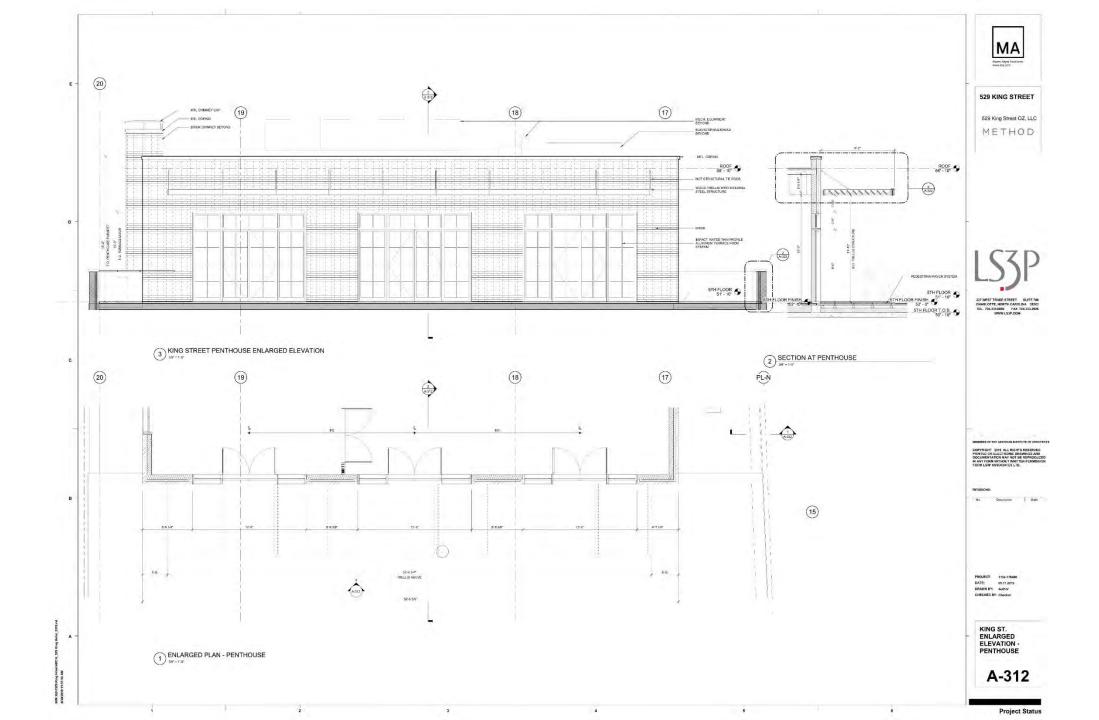


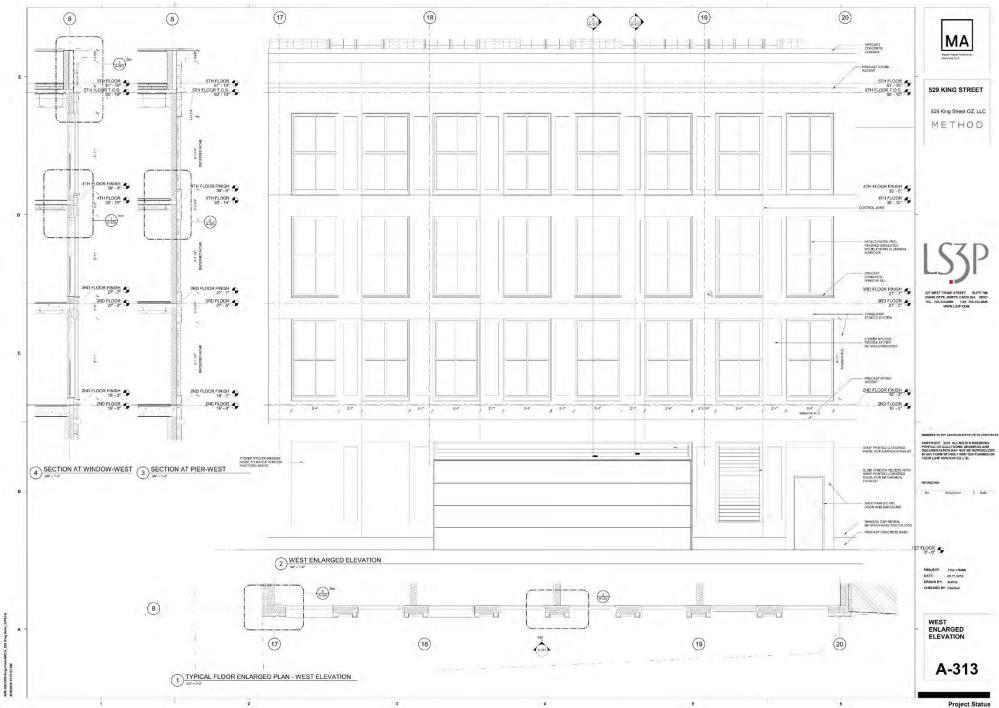
**Project Status** 

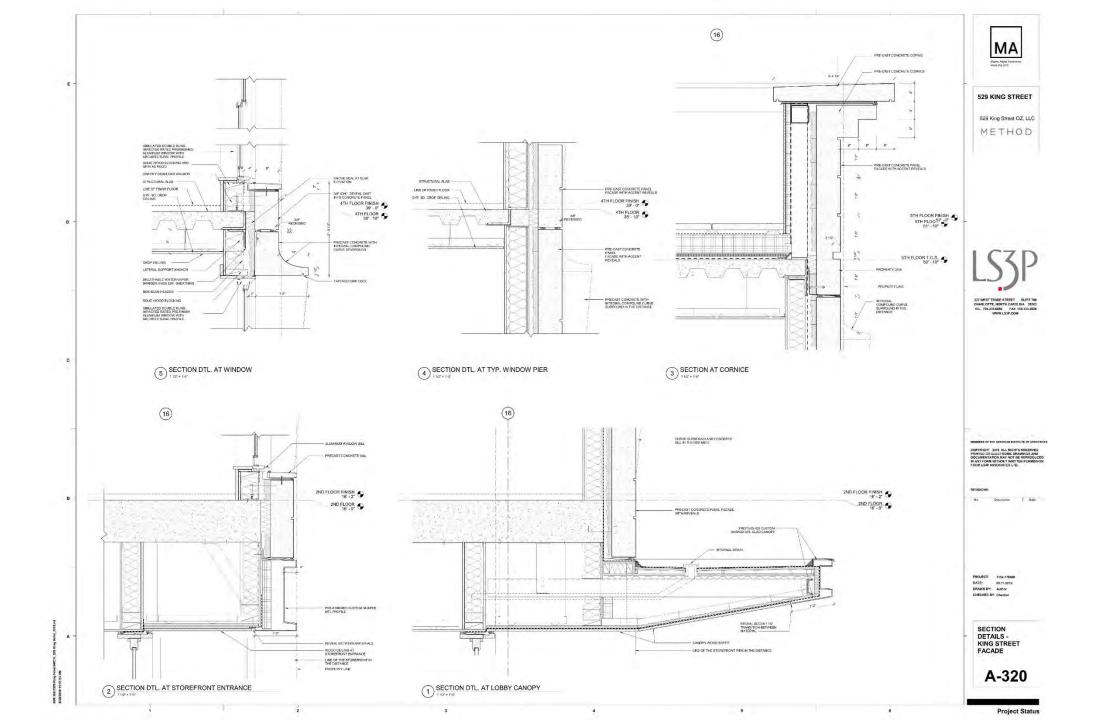


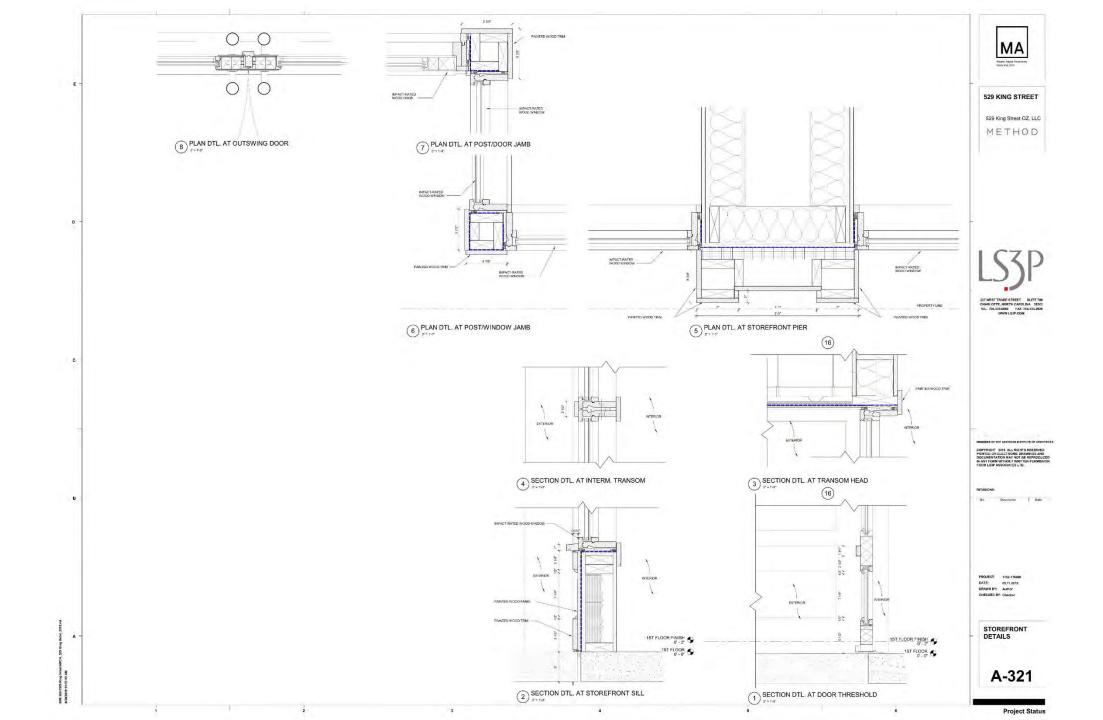


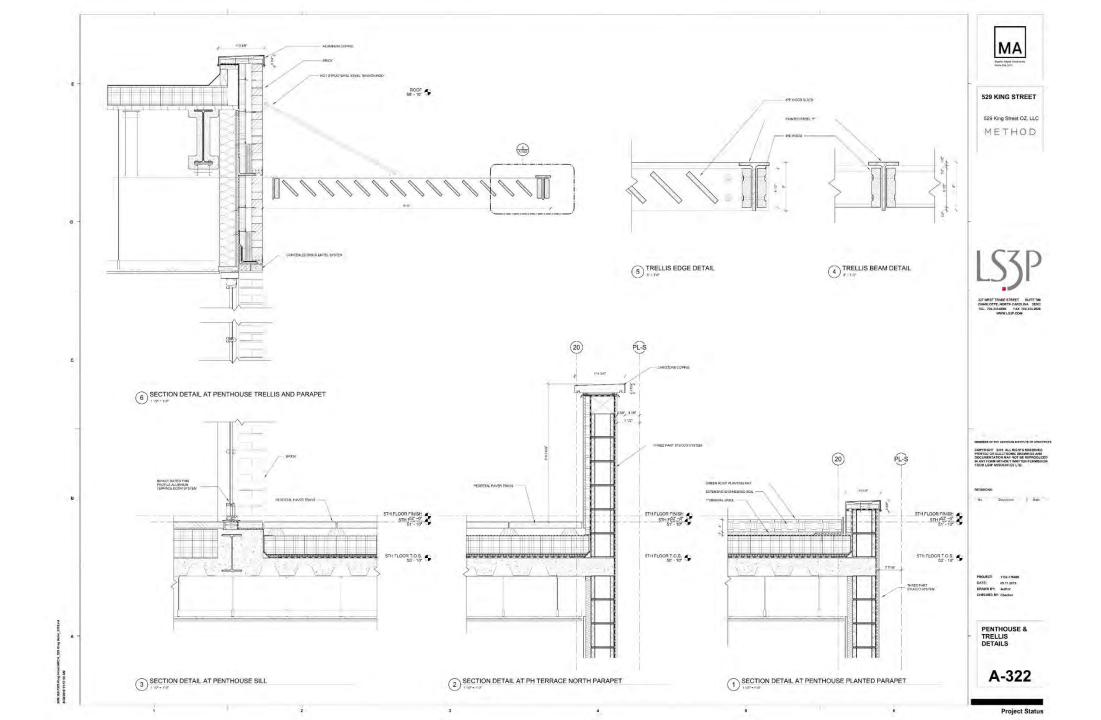


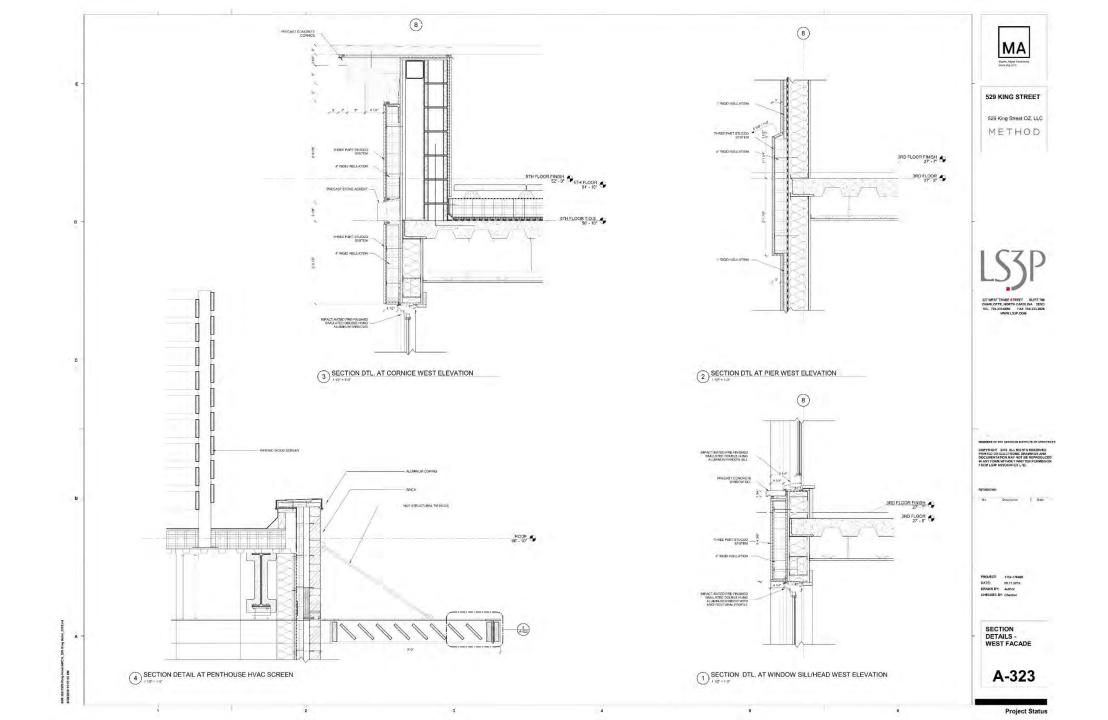


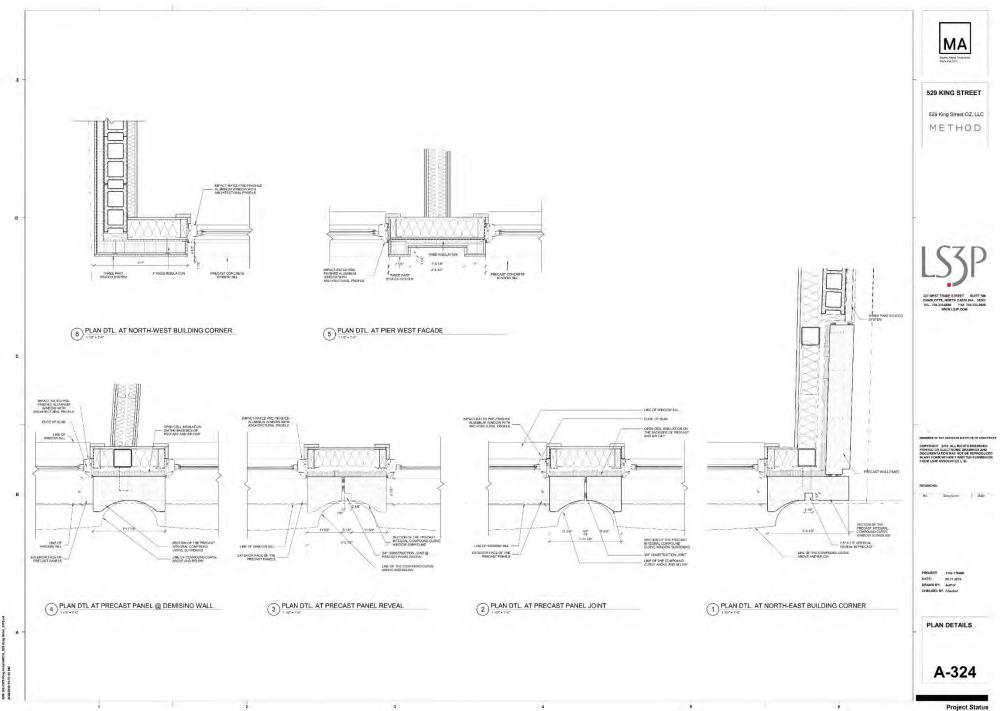












Agenda Item 7:

## 741 Meeting Street - - TMS #463-12-02-020, -021, -022

Request conceptual approval for new construction of mixed-use building and requesting an additional half-story for architectural merit and context.

(East Central) / Historic Corridor District

# 741 MEETING STREET

### CHARLESTON, SOUTH CAROLINA



Board of Architectural Review - Conceptual Design Summer 2019

Amended September 2019

### PREFACE

The following manual presents the conceptual design for a new mixed-use commercial building at 741 Meeting Street in Charleston, South Carolina. Materials were developed during a design charrette held in July of 2019.

The subject site includes a direct frontage on Meeting Street, at a rare cross-grain terminus at the western end of Williman Street, This transitional portion of Meeting Street contains a mixture of residential and industrial or auto-oriented commercial uses, many of which are no longer in operation. It is also a transitional area, with the larger scaled urban industrial character of the upper peninsula to the cast, a lower scaled residential neighborhood character to the north and south, and the elevated Highway 26 to the immediate west.

This transitional character presents an opportunity to create a timeless, durable architecture that acknowledges both the industrial and residential character of its context while re-introducing street level commercial uses that are currently lacking in this neighborhood.

This document incudes historical and site analysis, a site conditions study, conceptual master plan and the conceptual design for the project's architectural expression.

SUMMER 2019

741 MEETING STREET

Urban Redevelopment Analysis

Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Cluic Architecture* 

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741 MEETING STREET

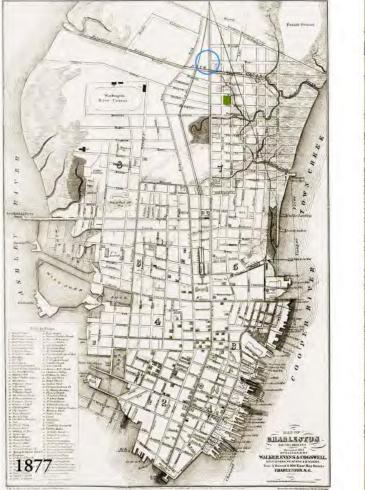
Urban Redevelopment Analysis

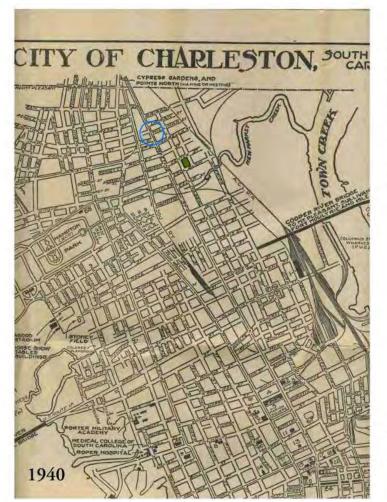
## REDEVELOPMENT AREA

District Evolution Upper Peninsula District Context Redevelopment Site Context Context Photo Study Site Survey

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Urban Redevelopment Analysis





#### TRANSITIONAL CHARACTER

The site is located in the upper peninsula along Meeting Street at the western terminus of Williman Street. Over the course of the late 20th century, the Meeting Street frontage has been eroded by auto oriented and industrial commercial uses with parking placed between the buildings and the streets while neighborhood oriented, street level commercial uses are largely non-existent.

The site is locateed at a rare cross-grain terminus in the urban plan that captures long views down Williman Street, similar to the site of the Trolley Barn at the terminus of Cool Blow Street to the north.



The Tiolley Barn is an example of early industrial architecture creating an anchor on upper Meeting Street.

#### 741 MEETING STREET

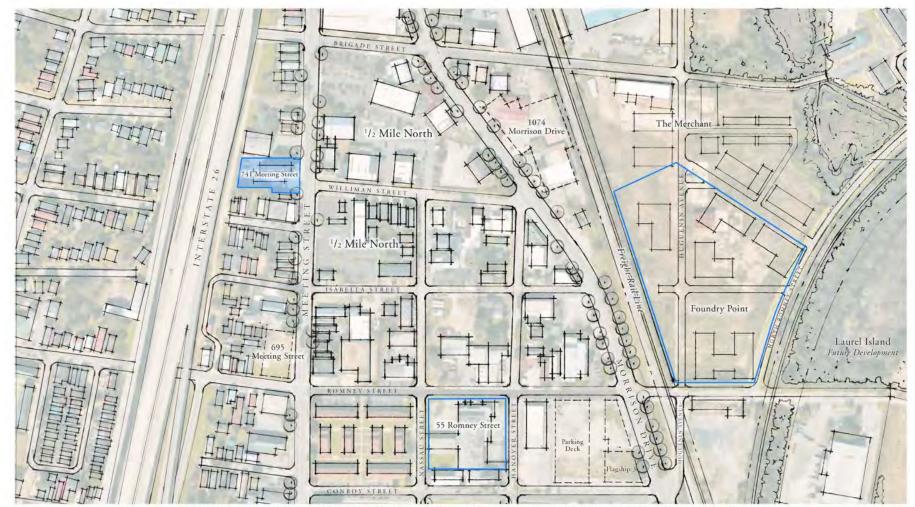
### Urban Redevelopment Analysis

Middle Street Parinets, LUC The Middleton Group, LUC Sottile & Sottile, *Civit Architecture* 

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#### 741 Meeting Street | Charleston, South Carolina

DISTRICT EVOLUTION



The 741 Meeting Street Site is situated in an area of the upper peniasula under active redevelopment with close proximity to projects currently in-progress including Foundry Point and 55 Romney Street.

DISTRICT CONTEXT

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Meeting Street & Surrounding Projects

741 MEETING STREET Urban Redevelopment Analysis

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An aerial view of the Upper Peninsula District and its relationship to the 741 Meeting Street vite.

Information contained herein is conceptual, Information has here compiled from criticus sources and does not claim complete accuracy une guarantee musing or other types of development approvals. It is intended to provide an overview and authoris of urban conditions and strategies fur revisalization. Sortile & Sortile, 2019 DISTRICT CONTEXT

741 MEETING STREET

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA Middle Steert Partners UP

Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Civic Architecture* 

Meeting Street & Surrounding Projects

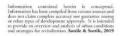




Approaching the site on-axis from the east on Williman Street.



The Maeting Street frontage is a continuous curb-cut with no defined sidewalk.



SITE CONTEXT

#### 741 Meeting Street | Charleston, South Carolina



View from Meeting Street looking northwest.



Looking west next to existing structure with mature trees along property line.

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Neighboring buildings to the north: a vacant home and concrete block commercial building.



View approaching the site from the south on Meeting Street.



View down the northern frontage of Williman Street directly across from the site.



## SITE CONTEXT

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741 Meeting Street | Charleston, South Carolina



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Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Civic Architecture* 



4 Maple Street, a permitted three and a half story office building to the north of the project site.



Existing corner commercial and institutional uses adjucent to the project site.



Open parking and Half-Mile North commercial complex across Meeting Street from the project site.



View approaching the site on the elevated 1-26 overpass.

## SITE CONTEXT

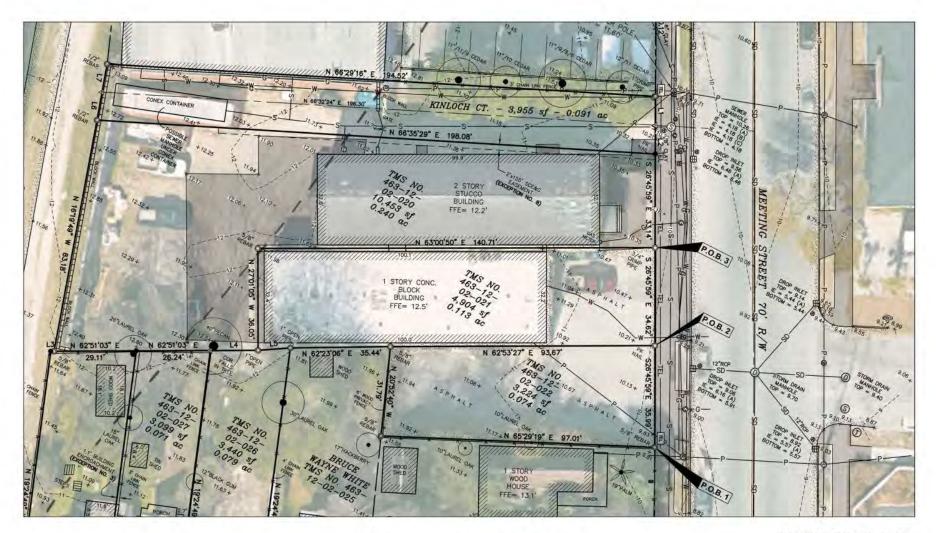
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741 Meeting Street | Charleston, South Carolina

741 MEETING STREET

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### SITE SURVEY

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741 Meeting Street | Charleston, South Carolina

741 MEETING STREET

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA Middle Street Pariners, LLC The Middleton Group, LLC South & South, Crine Architeance

## DESIGN CHARRETTE

Schedule & Kick-Off Meeting Site & Precedent Tour Planning Concepts Architectural Design Concepts

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Urban Redevelopment Analysis



Sottile & Sottile Urban Design & Civic Architecture



clarified urban challenges along Meeting Street.



The design team explored multiple options through a zeries of feedback sessions with the development team and stakeholders.



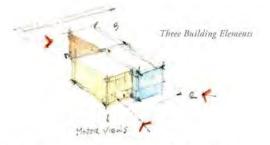
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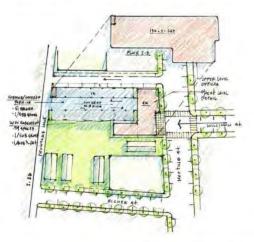
741 Meeting Street | Mixed-Use

#### DESIGN CHARRETTE

The design and development team organized a design charrette near the project site in July of 2019 to develop initial conceptual planning and architectural concepts for the site at 741 Meeting Street. Key ideas revolved around the orientation of the site as the terminated view of the west end of Williman Street.



A series of initial massing and site design studies focused the master plan on the alignment of the Williman Street axis.



#### 741 MEETING STREET Urban Redevelopment Analysis CHARLESTON. SOUTH CAROLINA Middle Street Parners, LU The Middleton Group, LU Souti & South, Cinie Arbiteurer



**RESTORING STREET FRONTAGES -**The existing buildings are set back with gravel lots and a continuous curb-cut along the entire frontage of the property. The design team explored building formats that would restore the sidewalk and address viewsheds around the site.



The design team discussed ways to restore the streetscape along Meeting Street and how to address the overpass at the rear of the property.





The historic Trolley Barn caps the view west on Cool Blow Street in a similar manner.



CAPTURING VIEWS Williman Street terminates at the west into the project site, setting up an opportunity for the new building to compliment the view created on axis with Foundry Point.

#### 741 MEETING STREET

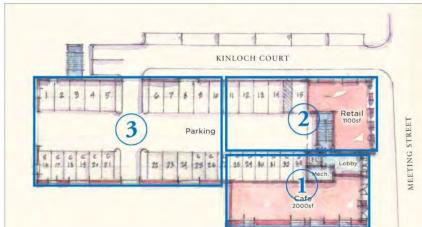
Urban Redevelopment Analysis

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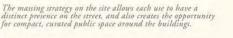
# DESIGN CHARRETTE

Site Tour & Planning Concepts



### Three Building Elements CONCEPTUAL MASSING -CAFE | RETAIL | OFFICE









The proximity of nearby residential forms began to influence the desire to create a more expressive roof massing.

### Complimentary Elements

The location of the site along Meeting Street, a redeveloping mixed-use corridor, but near adjacent bistoric residential properties, led the design team to consider a massing strategy that broke the scale of the commercial building into multiple, offset masses, much like traditional mixed-use development along nearby King Street.

# DESIGN CHARRETTE

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### Massing Concepts

#### 741 MEETING STREET

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Architectural Concepts

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## ARCHITECTURAL CHARACTER



The design concept focuses on the consistent use of a delicate, metal window with detailed profiles in the surrounding masonry to provide depth and architectural detail to the facades.

## DESIGN CHARRETTE

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Architectural Concepts





The Preservation Society of Charleston provided input on local context and building patterns that influenced the architectural direction for the building. Concepts to create a unique building that complements the surrounding district were a focus of the design feedback sessions.



#### 741 MEETING STREET

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA

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The concept for the architectural design prioritzes the urban street and the pedestrian space, while presenting a well-mannered, but restrained, facade to the highway overpass.

> The architectural massing transitions from an expressed gable to a low parapet as it returns to the interstate, emphasizing the building's priority towards Meeting Street, and creating the dialogue of a warehouse massing to the rear of a street-fronting commercial building.

### DESIGN CHARRETTE

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Conceptual View from I-26

#### 741 MEETING STREET

Urban Redevelopment Analysis



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Conceptual View from Williman Street

#### 741 MEETING STREET

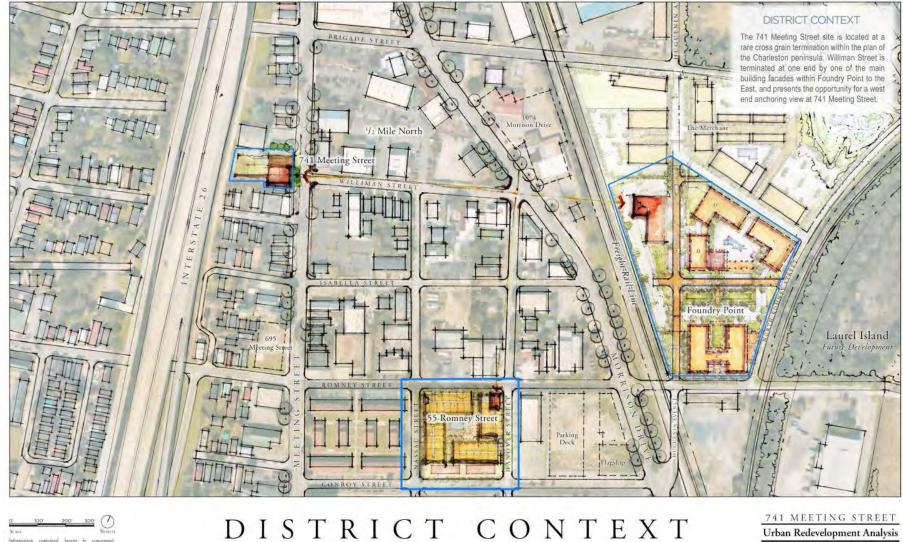
Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA

## MASTER PLAN

District Context Redevelopment Master Plan Street Level Engagement Conceptual Massing Context Massing Conceptual Views

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Meeting Street & Surrounding Projects

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA Middle Street Pariners, LIC The Middleton Group, LIC Sonile & Sonile, *Cinic Architecture* 21



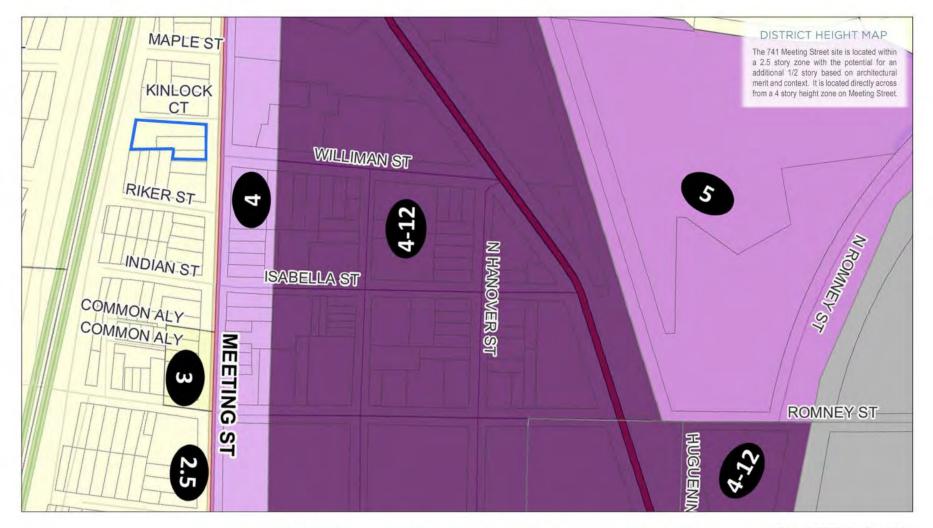
# COMMERCIAL ZONING

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Meeting Street & Surrounding Parcels

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Urban Redevelopment Analysis



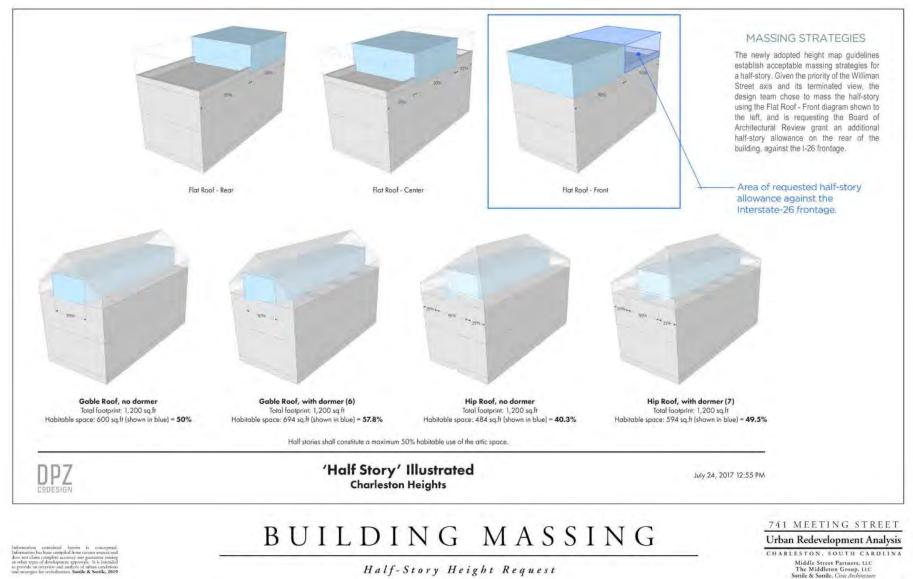
## DISTRICT HEIGHT MAP

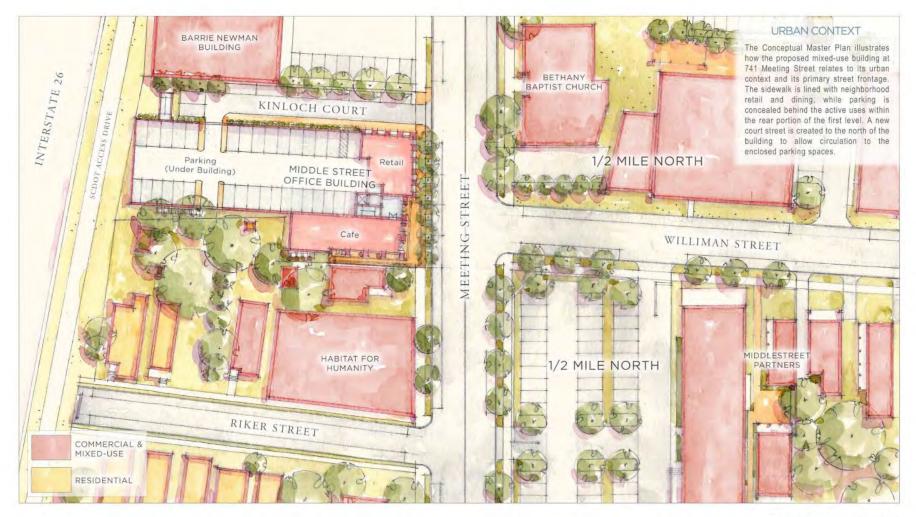
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# CONCEPTUAL MASTER PLAN

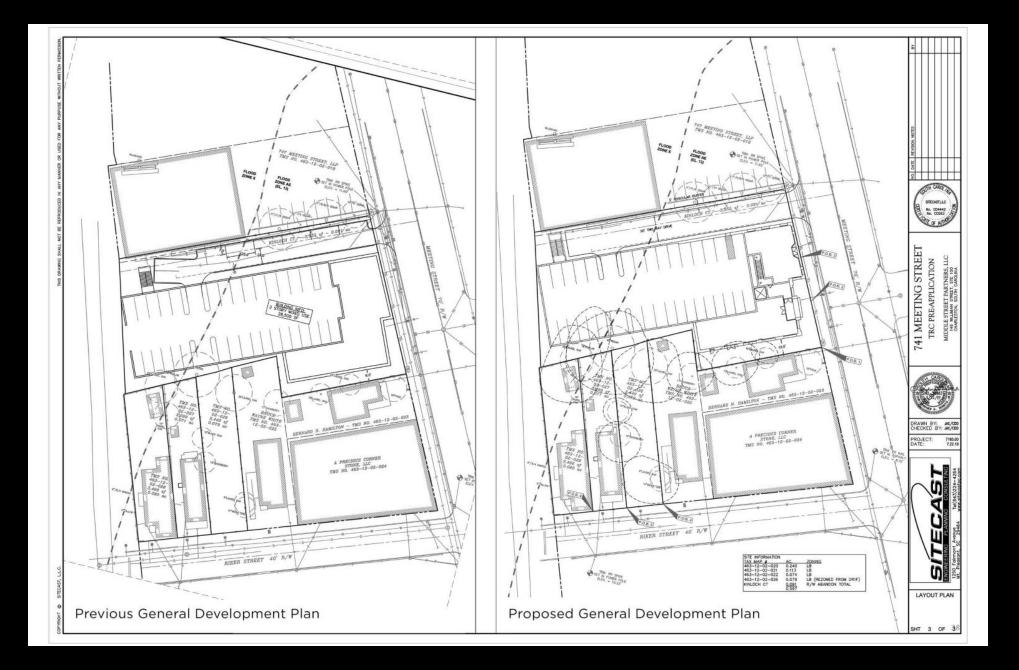
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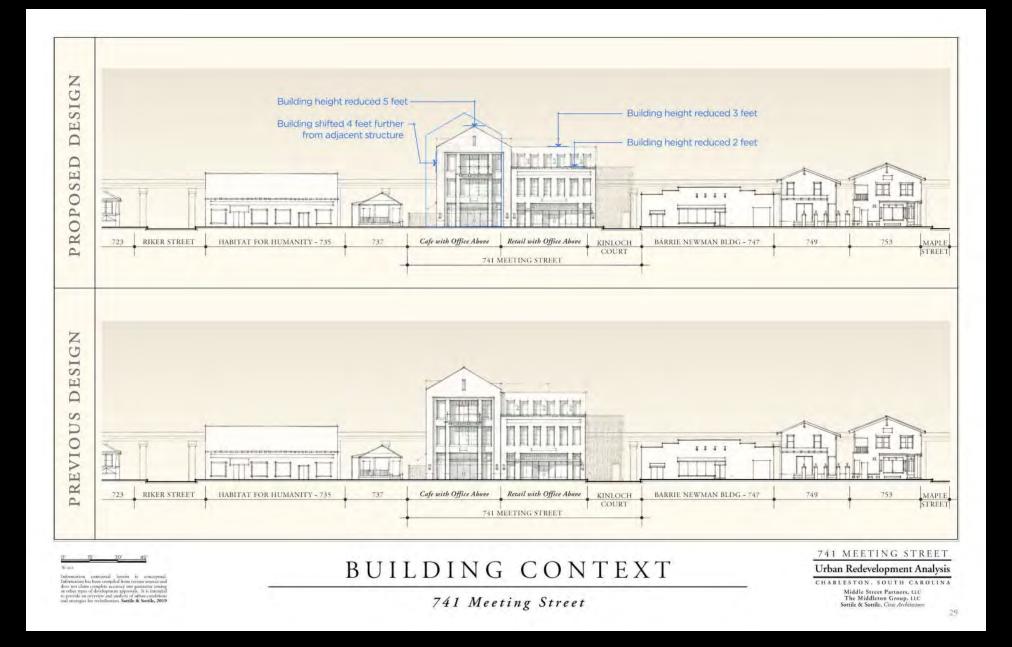
741 Meeting Street

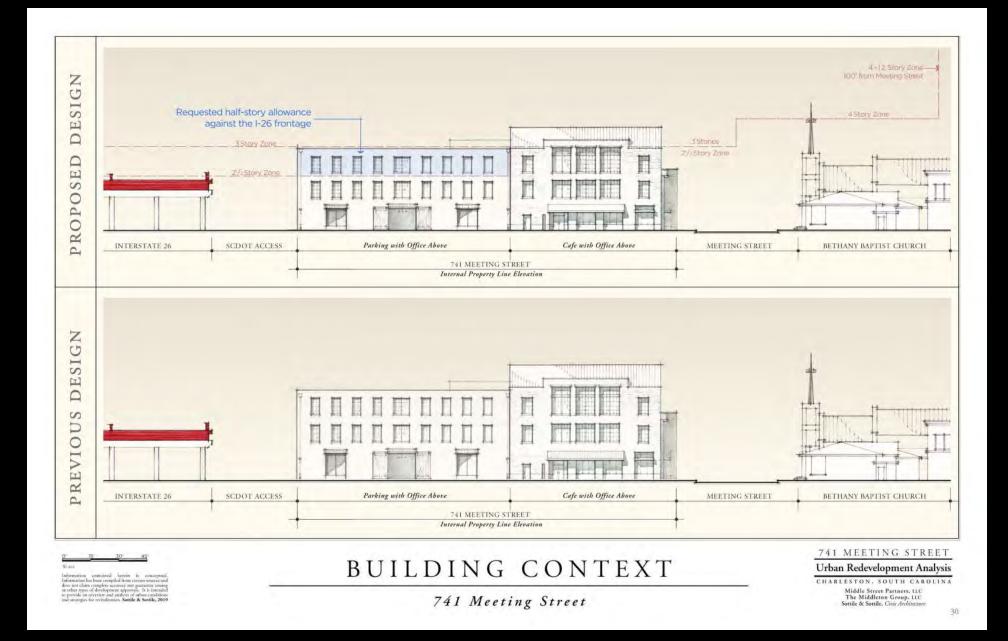
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741 Meeting Street

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# 741 MEETING STREET

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741 Meeting Street

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### Urban Redevelopment Analysis

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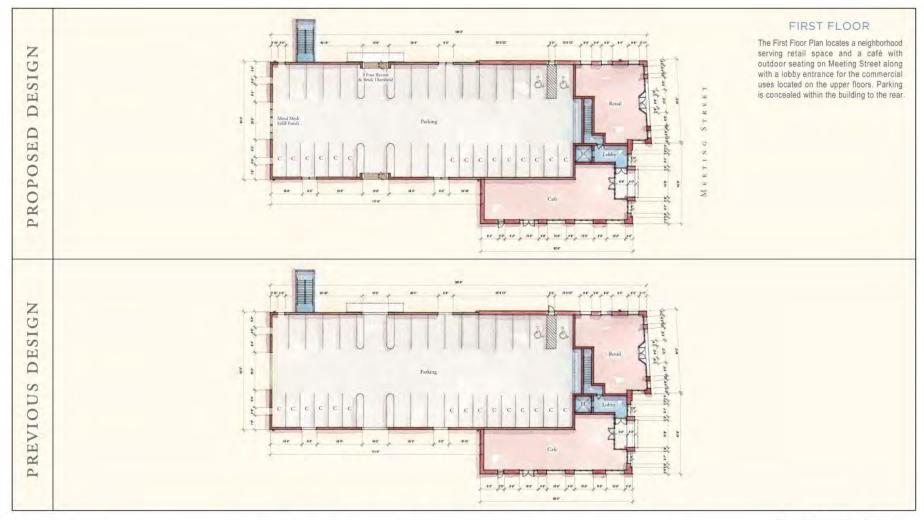
# ARCHITECTURAL CONCEPTS

Conceptual Floor Plans First Floor Second Floor Third Floor Roof Plan

Conceptual Elevations Meeting Street Elevation Kinloch Court Elevation South Elevation West Elevation

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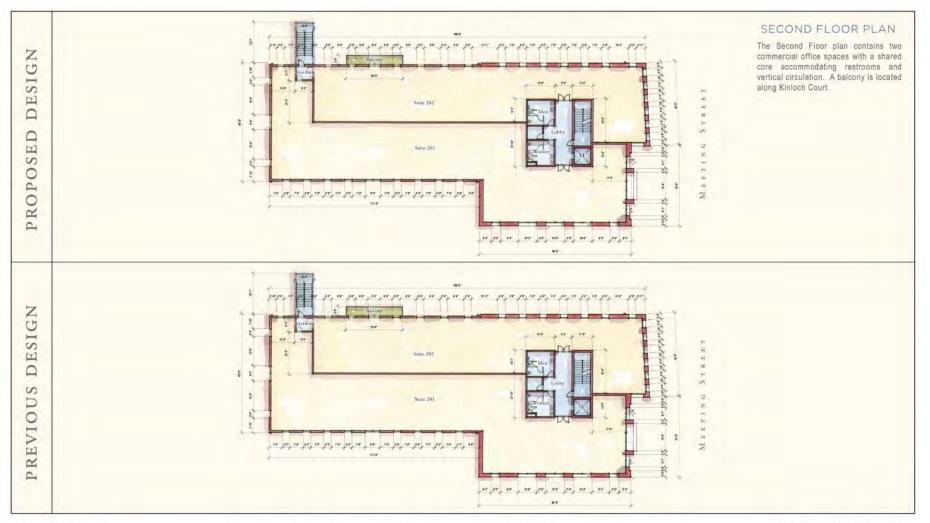
### CONCEPTUAL FLOOR PLANS

741 MEETING STREET

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Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Civic Architecture* 34

Mixed-Use Office and Retail Redevelopment



0' 10' 20' Scala

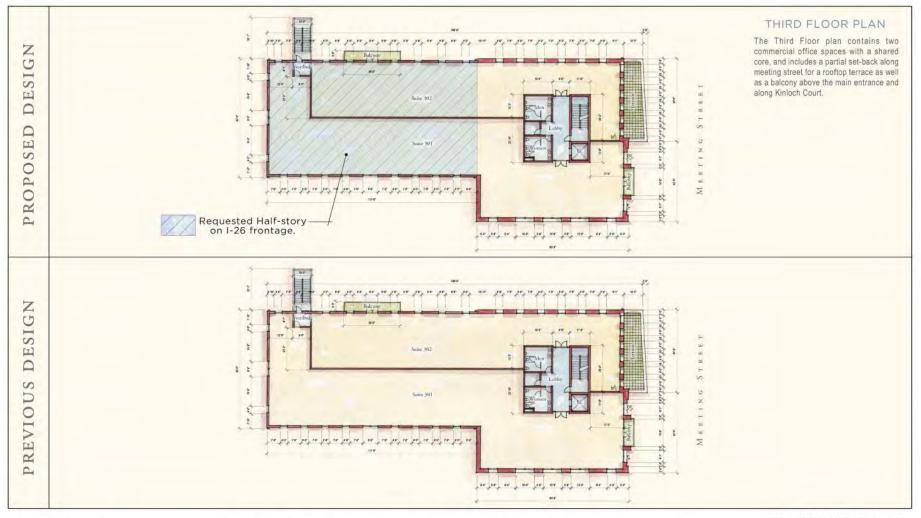
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### CONCEPTUAL FLOOR PLANS

Mixed-Use Office and Retail Redevelopment

#### 741 MEETING STREET

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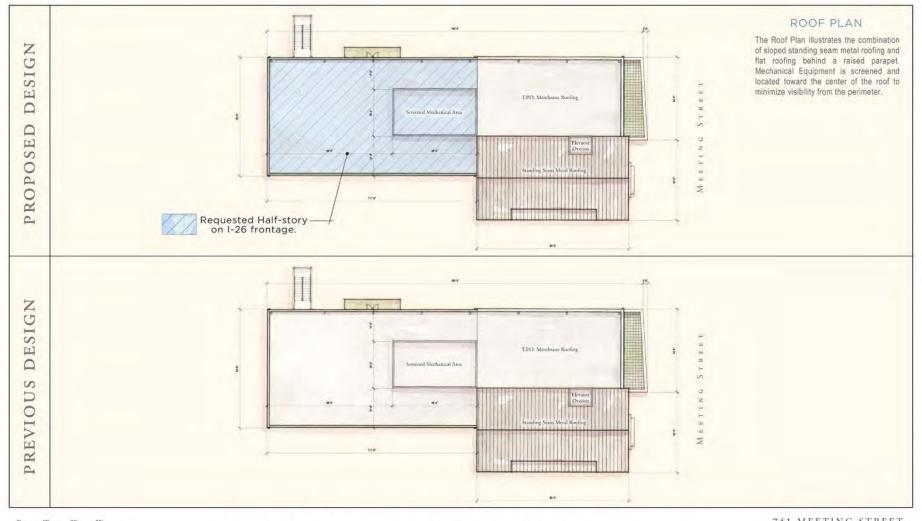
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Mixed-Use Office and Retail Redevelopment



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741 MEETING STREET

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA

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Mixed-Use Office and Retail Redevelopment

# - Conceptual Elevations —

Meeting Street Elevation Kinloch Court Elevation South Elevation West Elevation

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741 Meeting Street

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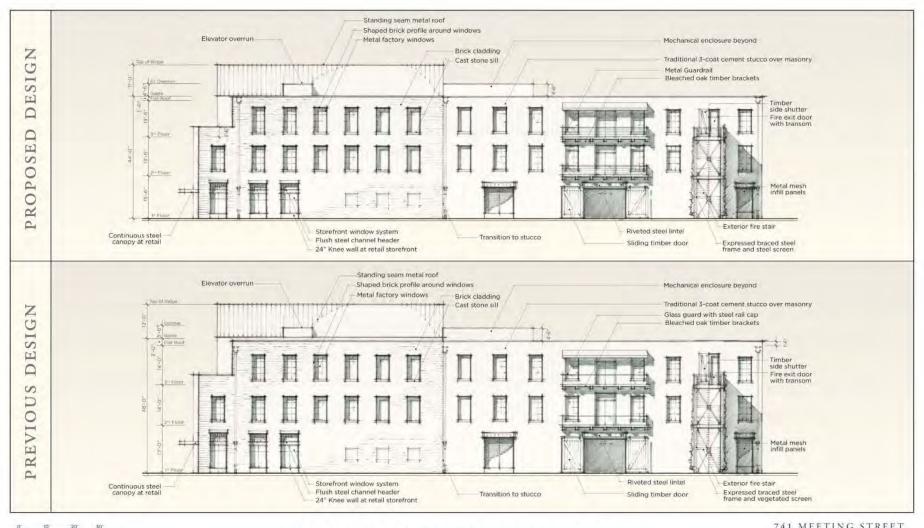
Mixed-Use Office and Retail Redevelopment

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741 Meeting Street



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741 MEETING STREET

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, Civic Architecture 42

Mixed-Use Office and Retail Redevelopment



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741 Meeting Street

CHARLESTON, SOUTH CAROLINA



Mixed-Use Office and Retail Redevelopment

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741 Meeting Street

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CONCEPTUAL ELEVATION DETAILS

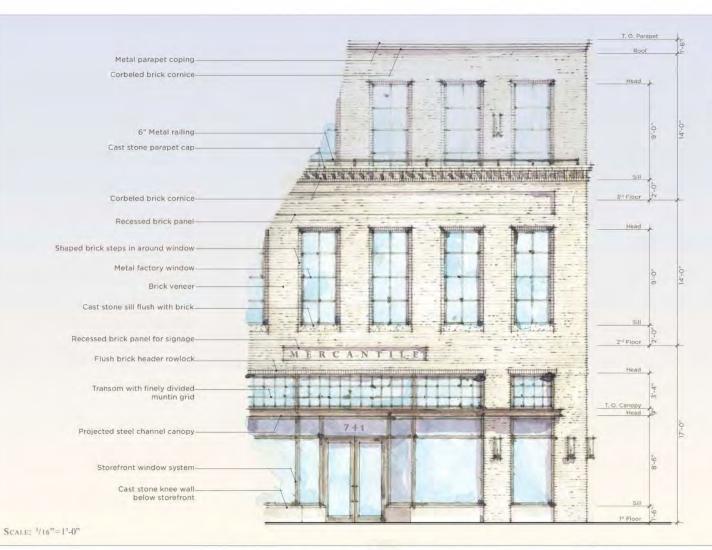
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CONCEPTUAL ELEVATION DETAILS

Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA



### RETAIL FRONTAGE

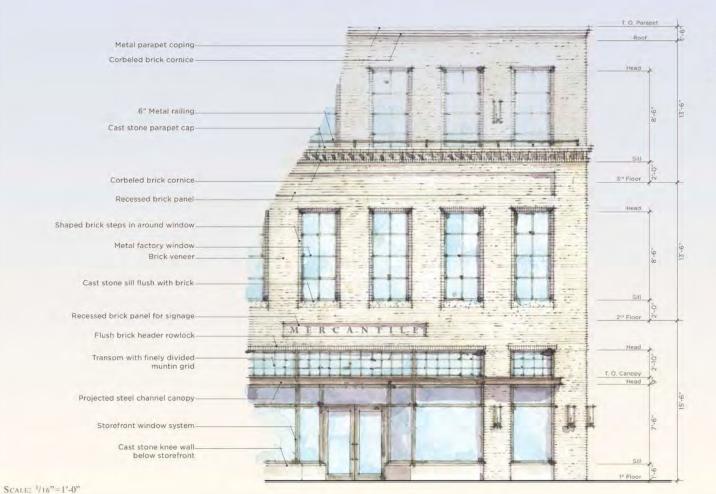
This elevation detail illustrates the retail storefront on Meeting Street, and the upper level windows and cornice expression, as well as the third level façade, which is set back to create a roof terrace.

Information contained barein is conceptual, Information has been compiled from various sources and does not ealine complete accuracy nor guaraneous raming or other types of development approvals. It is intended to provide an orecrises and analysis of urban conditions and strategies for restriktington. Sortile & Sortils, 2019

### CONCEPTUAL ELEVATION DETAILS

741 MEETING STREET Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA

Previous Design



#### RETAIL FRONTAGE

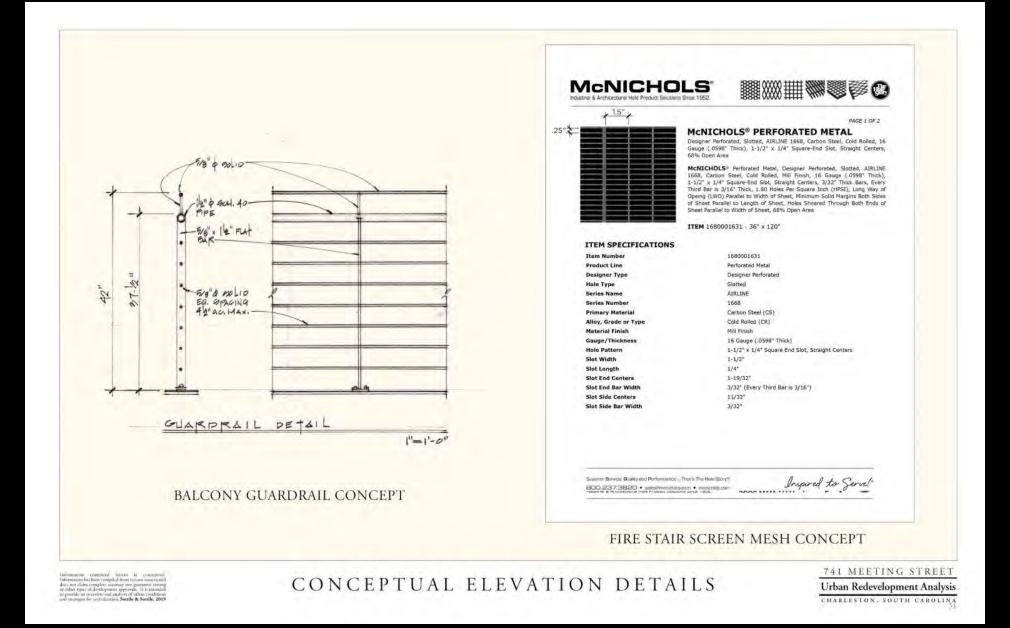
This elevation detail illustrates the retail storefront on Meeting Street, and the upper level windows and cornice expression, as well as the third level façade, which is set back to create a roof terrace.

Proposed Design

Information contained berein is conceptual, Information has been compiled from various sources and does not ealine complete accuracy nor guarance running or other types of development approvale. It is intended to provide an orretrieve and mathesis of urban conditions and strategies for restrictingation. South & Souths, 2019

### CONCEPTUAL ELEVATION DETAILS

741 MEETING STREET Urban Redevelopment Analysis



### PATTERNS & PRECEDENTS

Massing and Details Exterior Spaces and Site Design Interior Spaces

Information contributed literative is conceptual Information has been completed from regimes sources and does not show complete accurate net spacenet roming to other systes of development approach. It is intereded to provide at everytees and analysis of anther conditions and strategies for investiganous South & Souther, 2019 741 MEETING STREET

Urban Redevelopment Analysis



Massing and Details







Articulated brick detailing around openings in an otherwise minimal facade.

Painted brick combined with large windows.

Information contained herein is conceptual. Information has been compiled from crimon sonaxis and dues not claim complete assaying our guarantee running or other types of development approvals. It is intended to portice an corrective and antifysis of urban conditions and strategies for revisalization. Sottile & Sottile, 2019



An orderly arrangement of deeply set openings creates a strong play of light and shadow.



Simple, legible building massing and solid to void relationship.

PRECEDENTS PATTERNS &

#### 741 MEETING STREET

#### Urban Redevelopment Analysis CHARLESTON, SOUTH CAROLINA

Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Civic Architecture* 

Massing and Details

Information contained herein is conceptual. Information has been compiled from curious sonaxes and does not clinic complex accuracy (or guarantee running or other types of development approvals. It is intended to portice an corrective and antiples of orthan conditions and strategies for revisalization. Sottile & Sortile, 2019

Small interstitial spaces create intimate outdoor seating areas.

Exterior Spaces and Site Design

&

PATTERNS PRECEDENTS

Interior cafe seating that opens to an adjoining outdoor plaza Outdoor cafe seating adjoining a highly transparent facade.







CHARLESTON, SOUTH CAROLINA Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Civic Architecture* 

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Interiors rendered in natural materials and large windows.

Bright, naturally illuminated interior spaces.

Creative work spaces with a simple industrial character.



Natural, tactile interior finishes.

Information contained herein is conceptual. Information has been compiled from environs sonaces and does not clim complete assaying our guarantee running or other types of development approvals. It is intended to portice an corrective and antiples of urban conditions and strategies for revitalization. Sottile & Sostile, 2019



Cool and warm interior finishes create compelling contrast.

PATTERNS & PRECEDENTS

Warm interior of exposed brick, heavy timber and reclaimed wood.

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#### 741 MEETING STREET

### Urban Redevelopment Analysis CHARLESTON. SOUTH CAROLINA Middle Street Partners, LLC The Middleton Group, LLC Sottile & Sottile, *Civic Architecture*

Interior Spaces

Agenda Item 8:

175 Market Street- - TMS # 457-08-02-099

Request final approval for 8-foot screen wall to conceal cellular equipment on roof.

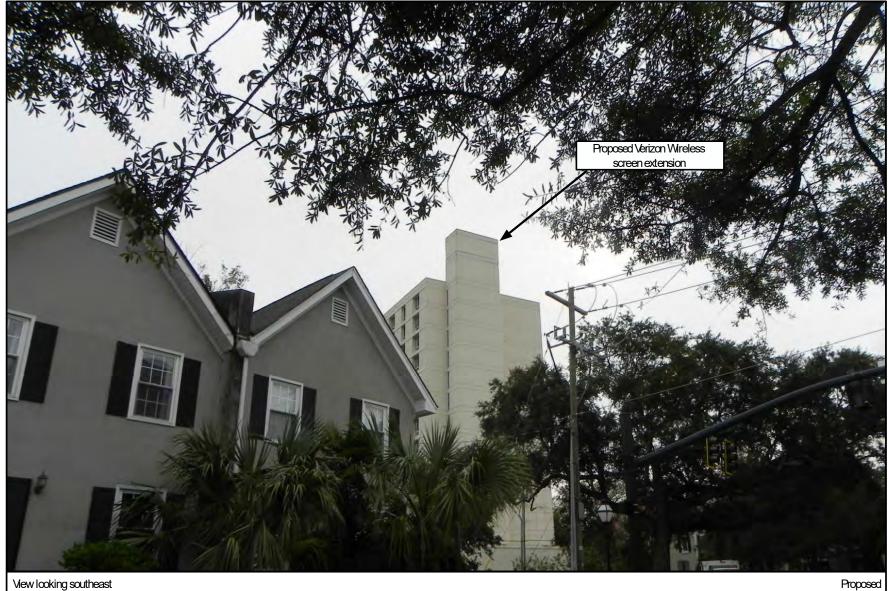
Not Rated / (Harleston Village) / c. 1970's / Old and Historic District



Kimley **»Horn** 

Verizon Wireless - Canterbury House 175 Market Street Charleston, SC 29401





View looking southeast



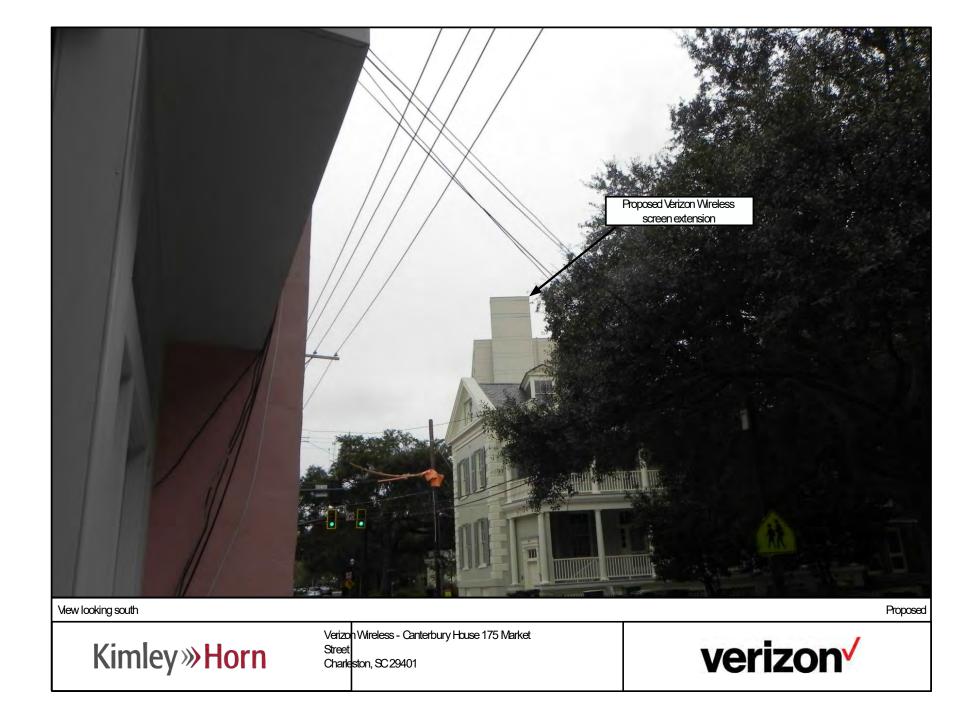
Verizon Wireless - Canterbury House 175 Market Street Charleston, SC 29401

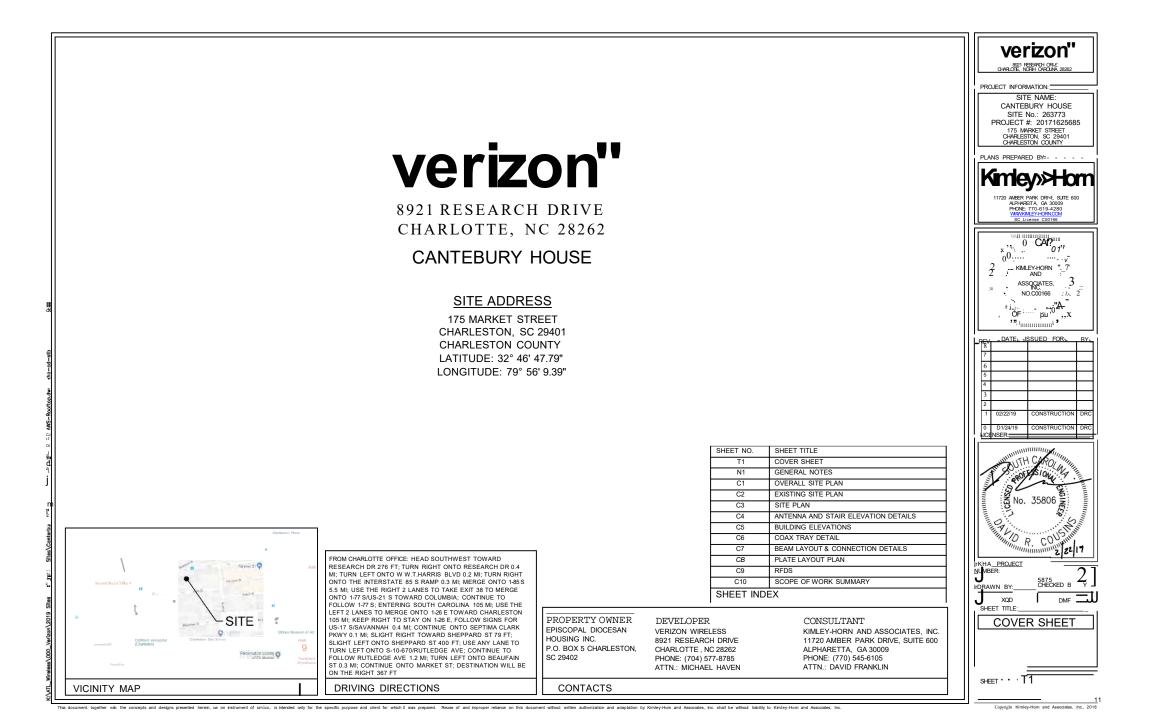










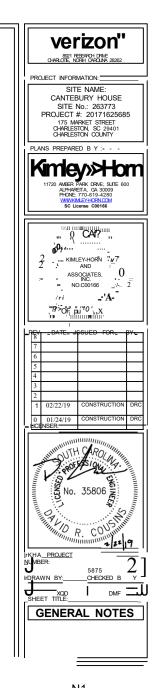


## 1.00 GENERAL NOTES

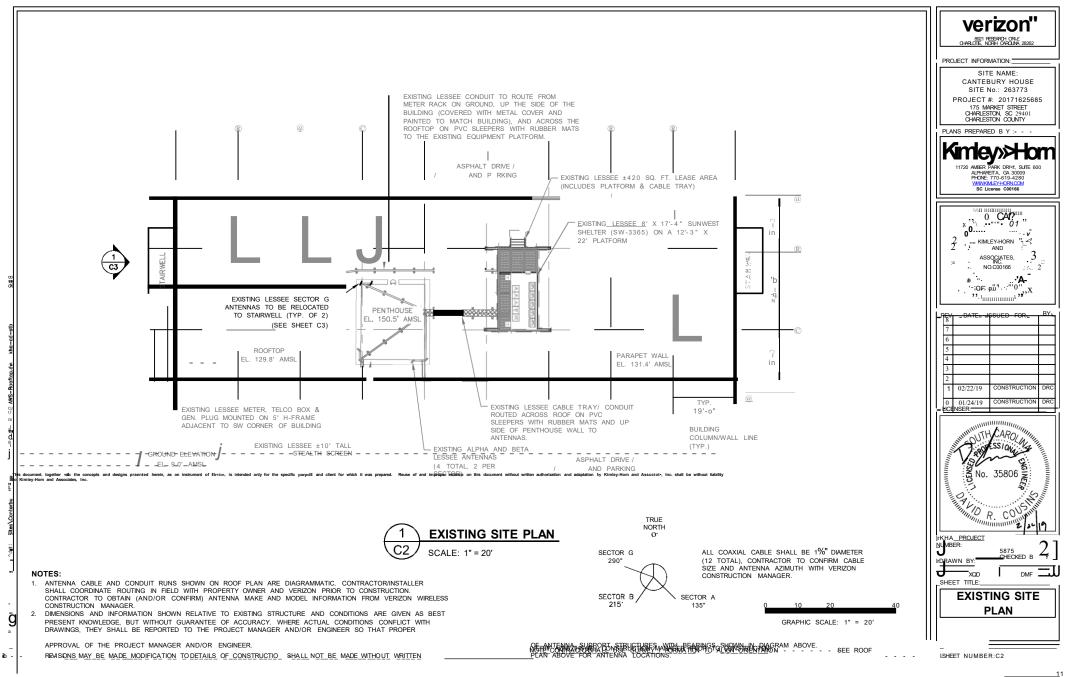
- ALL MATERIALS AND WORKWANSHIP SHALL CONFORM TO THE DRAWINGS AND SPECIFICATIONS. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE LATEST EDITION OF THE STATE, LOCAL AND NATIONAL CODES, ORDINANCES AND CR REGULATIONS APPLICABLE TO THIS PROJECT.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE PROJECT MWAGER AND/OR ENGINEER AND BE RESOLVED BEFORE PROCEEDING WITH WORK WHERE THERE IS A CONFLICT BETWEEN DRAWING AND VERZON SPECIFICATIONS, THE VERZON PROJECT BYGINEER SHOULD BE CONTACTED FOR CLARIFICATION.
- 1.03 ALL INFORMATION SHOWN ON THE DRAWINGS RELATIVE TO EXISTING CONDITIONS IS GNEN AS THE BEST FRAVILEDGE, BUT WITHOUT GUARANTEE OF ACCURACY. WHERE ACTUAL CONDITIONS CONFLICT WITH THE DRAWINGS, THEY SHALL BE REPORTED TO THE PROJECT MANAGER AND/OR ENGINEER SO THAT PROPER REVISIONS MAY BE MADE. MODIFICATION OF DETAILS OF CONSTRUCTION SHALL NOT BE MADE. WITHOUT WRITTENAPPROVAL OF THE PROJECT MANAGER AND/OR ENGINEER.
- 1.04 CONTRACTOR SHALL REVIEW AND BE FAMILIAR WITH SITE CONDITIONS AS SHOWN ON THE ATTACHED SITE PLAN AND/OR SURVEY DRAWINGS.
- 1.05 WAVEGUIDE BRIDGE AND EQUIPMENT CABINETS ARE SHOWN FOR REFERENCE ONLY. REFER TO SEPARATE DRAWINGS FOR SPECIFIC INFORMATION.
- 1.06 CONTRACTOR TO PROVIDE DUMPSTER AND PORTABLE TOILET FACILITY DURING CONSTRUCTION.
- 1.07 CONTRACTOR TO PROVIDE ANY NECESSARY SIGNAGE PER VERIZON PROJECT MANAGERS INSTRUCTIONS.
- UNLESS OTHERWISE INDICATED, VERIZON SHALL OBTAIN & PROVIDE CONSTRUCTION PERMITS. THE CONTRACTOR SHALL OBTAIN, AT HIS OWN EXPENSE, ALL REQUIRED LOCAL, CITY, STATE AND/OR COUNTY CONSTRUCTION LICENSES. UPON COMPLETION OF THE WORK THE CONTRACTOR SHALL APPLY FOR & PROVIDE A CERTIFICATE OF OCCUPANCY.
- 9. CONSTRUCTION WASTE MAY NEITHER BE BURNED NOR BURIED AND MUST BE TAKEN TO AN APPROVED LANDFILL.
- 1.10 SECURITY TO THE SITE SHALL BE MAINTAINED AT ALL TIMES.

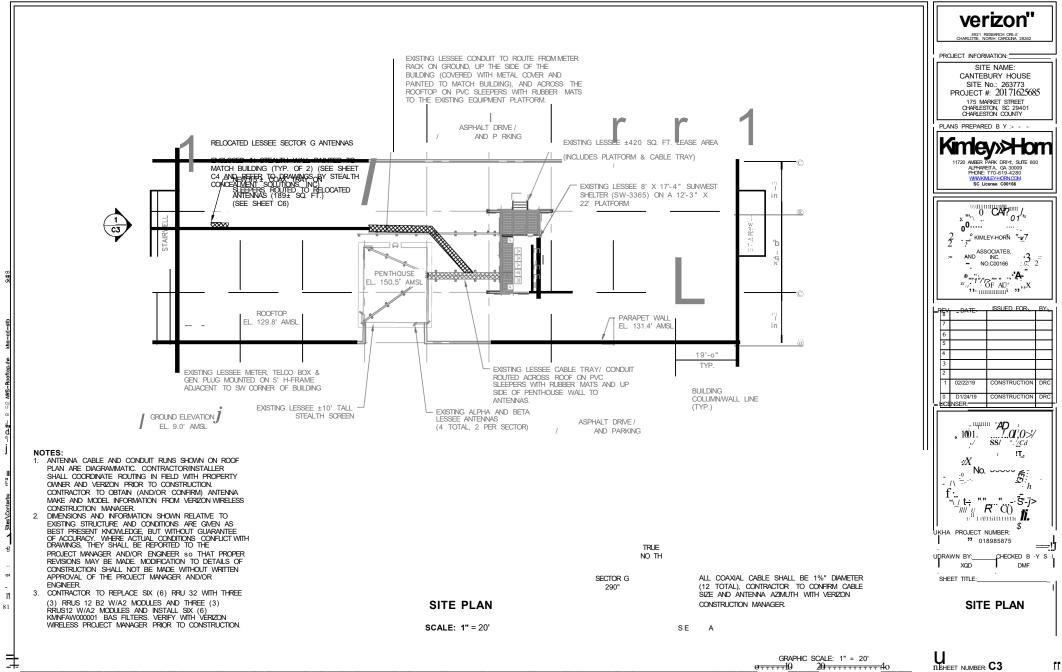
## 2.00 STRUCTURAL STEEL NOTES

- 2.01 STRUCTURAL STEEL SHALL CONFORM TO THE LATEST EDITION OF THE ASC "SPECIFICATION FOR THE DESIGN, FABRICATION & ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- 2.02 ALL INTERIOR STRUCTURAL STEEL SHALL BE FINISHED WITH ONE COAT FABRICATORS NON-LEAD, RED OXDE PRIMER PRIMING SHALL BE PERFORMED ATER SHOP FABRICATION TO THE GREATEST EXTENT POSSIBLE ALL DINGS, SCRAPES, MARS, & WELDS IN THE PRIMED AREAS SHALL BE REPAIRED BY FIELD TOUCH-UP PRIOR TO COMPLETION OF THE WORK
- 2.03 ALL EXTENDER STRUCTURAL STEEL SHALL BE GALVANZED IN ACCORDANCE WITH THE SPECIFICATION ASTM A123 UNLESS OTHERWISE NOTED GALVANIZING SHALL BE PERFORMED AFTER SHOP FABRICATION TO THE GREATEST EXTENT POSSIBLE. ALL DINGS, SCRAPES, MARS, & WELDS SHALL BE REPAIRED BY FIELD TOUCH-UP PRIOR TO COMPLETION OF THE WORK
- 2.04 HOLES SHALL NOT BE PLACED THROUGH STRUCTURAL STEEL MEMBERS EXCEPT AS SHOWN AND DETAILED ON THE DRAWINGS.
- 2.05 CONNECTIONS:
  - 2.05A ALL WEIDING SHALL BE DONE USING E70XX ELECTRODES AND SHALL CONFORM TO AISC AND AWS D1.1. WHERE FILLET WEID SZES ARE NOT SHOWN, PROVIDE THE MINIMUM SZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION", 13TH EDITION, AT THE COMPLETION OF WEIDING, ALL DAWAGE TO GALVANIZED COATING SHALL BE REPARED.
  - 2.05B BOLTED CONNECTIONS SHALL USE BEARING TYPE GALVANZED ASTM A325 BOLTS (3/4") AND SHALL HAVE A MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE.
  - 2.05C NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE 5/8" DIA GALVANIZED ASTM A307 BOLTS UNLESS NOTED OTHERWISE.
  - 2.05D CONNECTION DESIGN BY FABRICATOR WILL BE SUBJECT TO REVIEW AND APPROVAL BY ENGINEER.



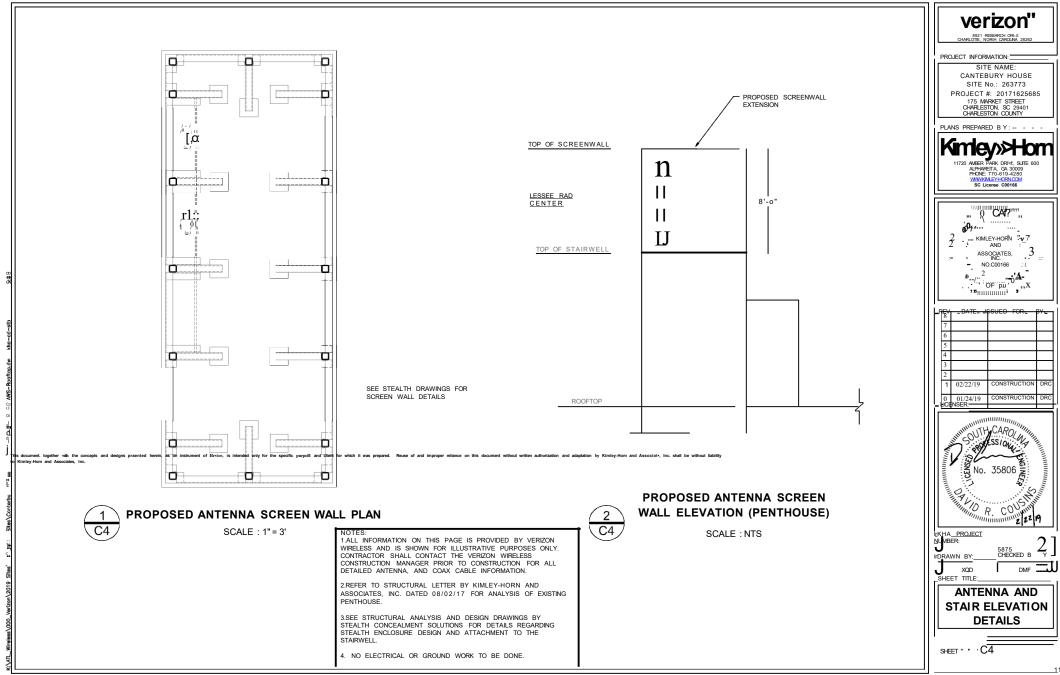




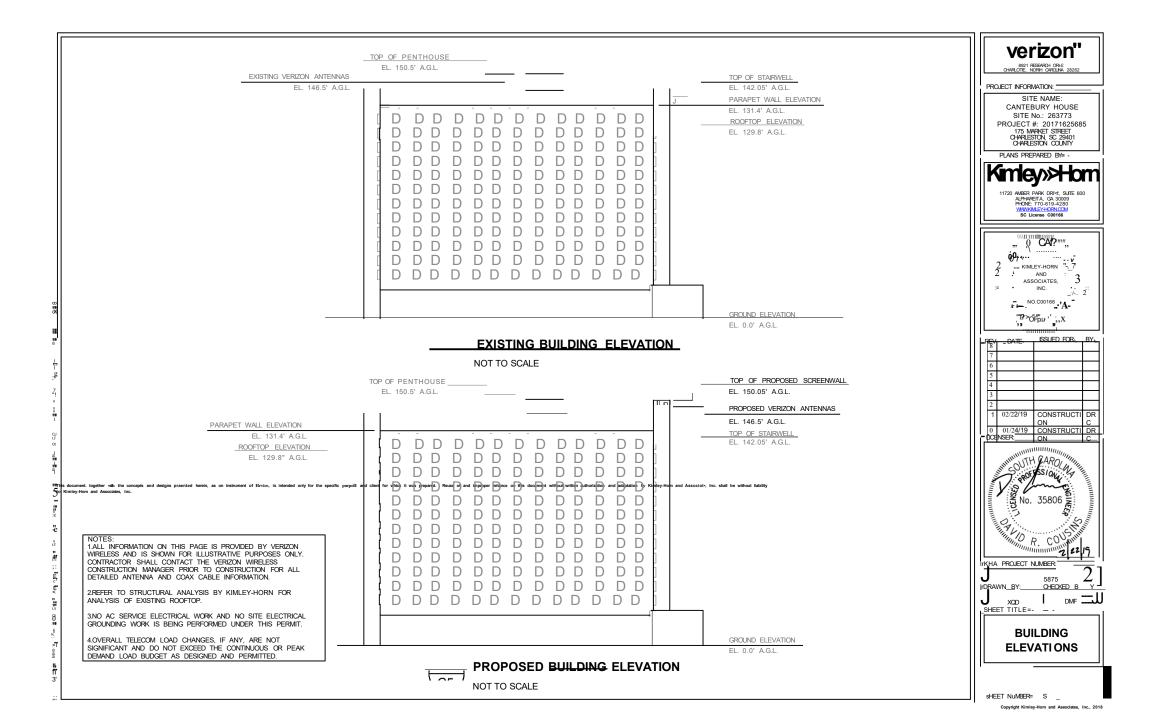


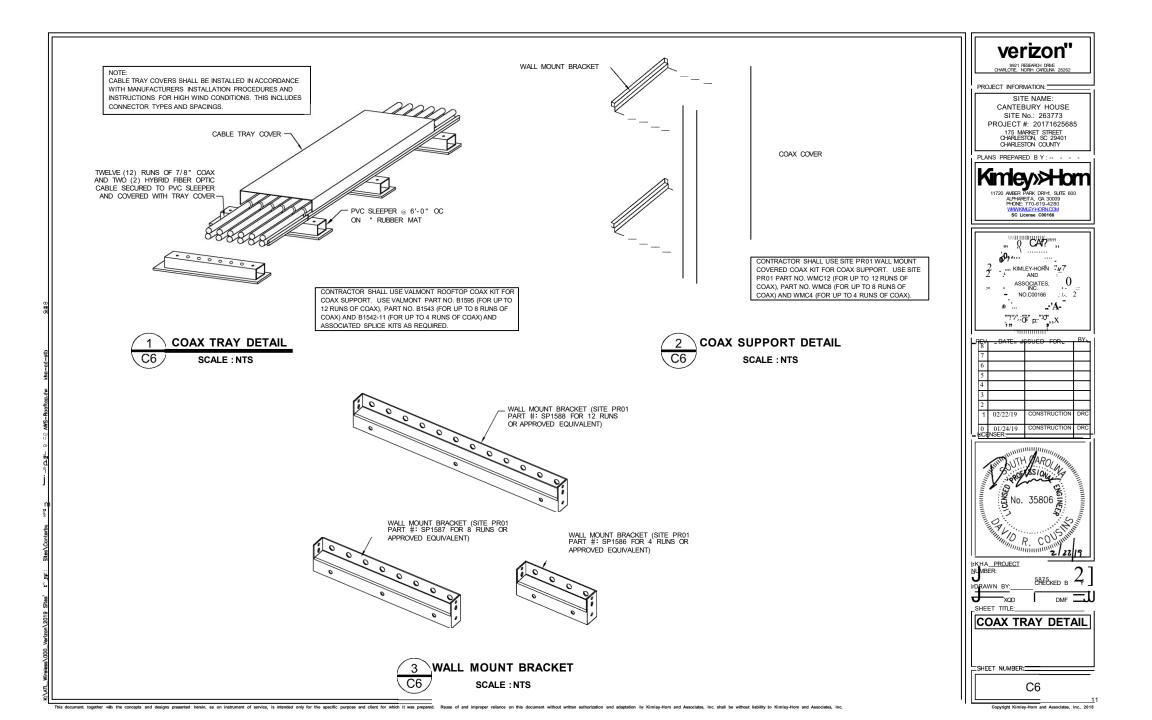
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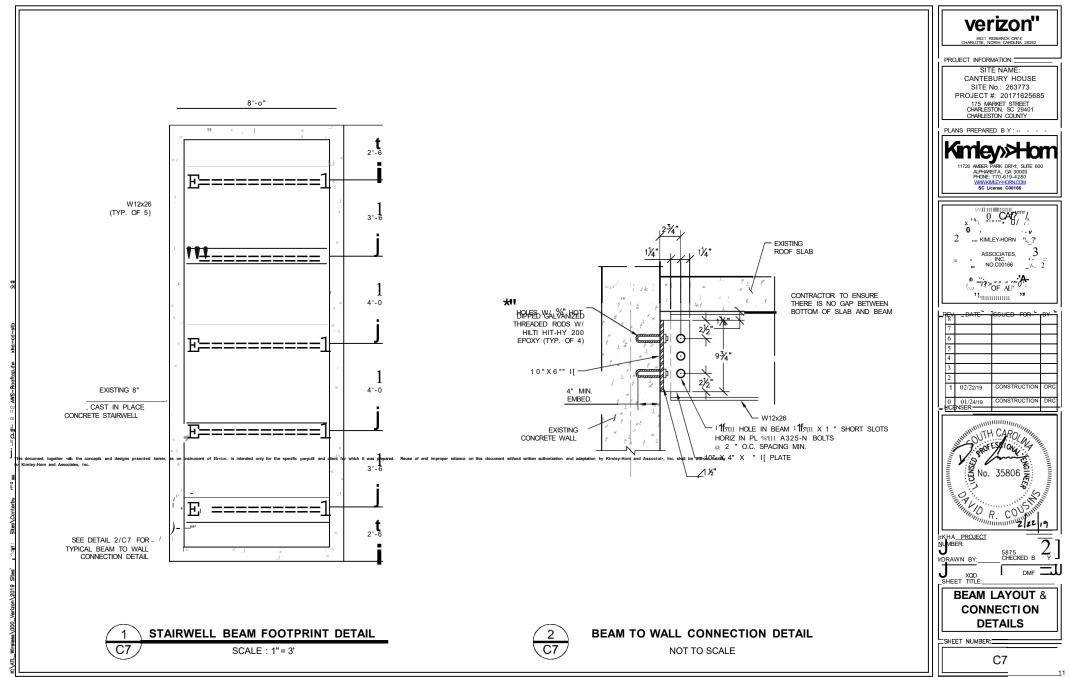




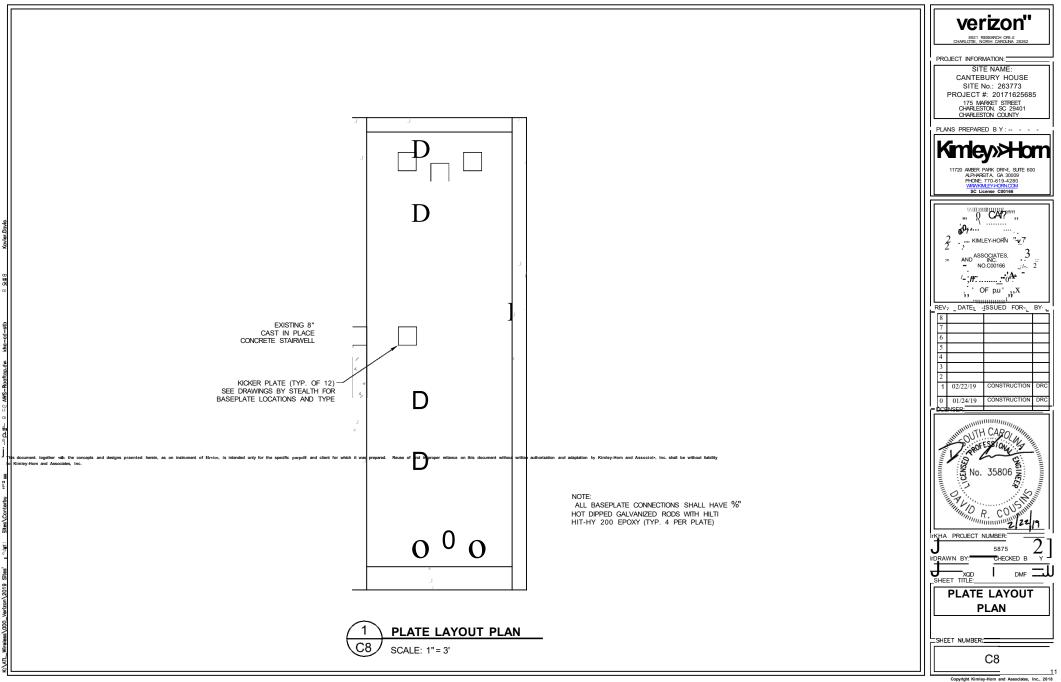
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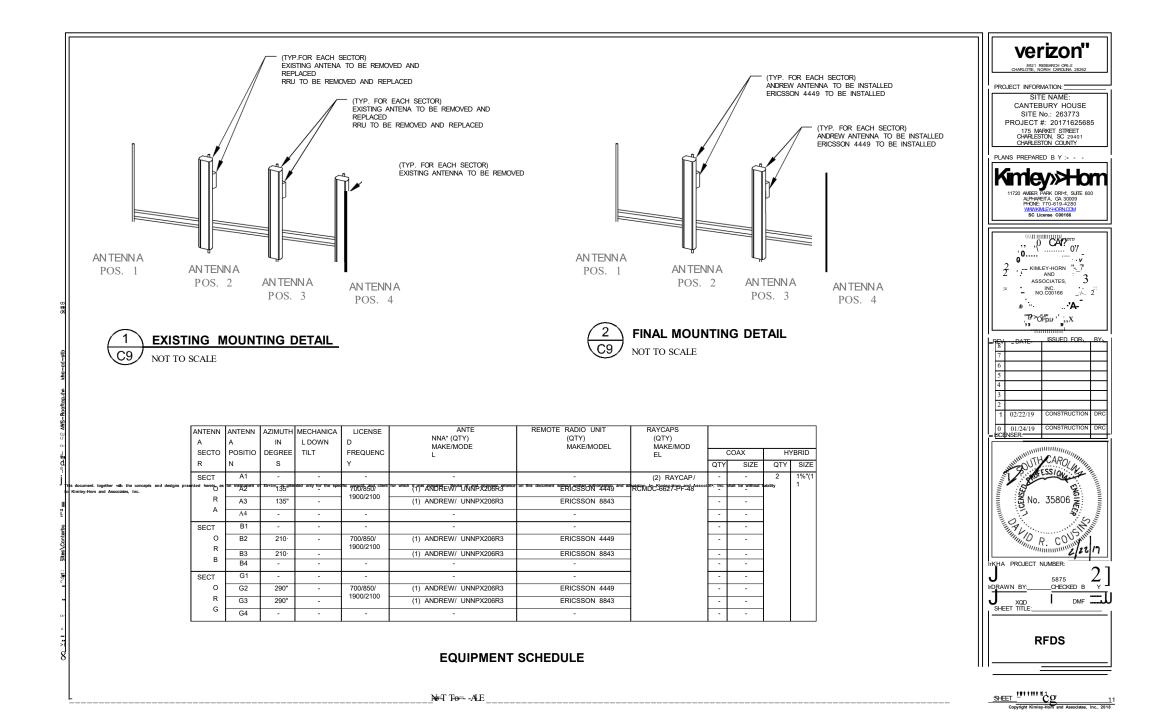


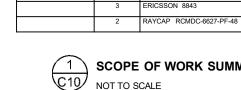




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| ١ | SCOPE OF WORK SUMMAR |
|---|----------------------|
| 1 | NOT TO SCALE         |

EQUIPMENT REPLACEMENT SUMMARY

DESCRIPTION

EXISTING EQUIPMENT

AMPHENOL WBX045T19ROOOG

CSS X7C-FR0-840-V

1-5/8" HYBRID FIBER

1-5/8" COAX (LEASE) 1-5/8" COAX

CSS X7C-FR0-840-V

ERICSSON RRUS 12 B4

ANDREW UNNPX206R3

1-5/8" HYBRID FIBER

1-5/8" HYBRID FIBER

1-5/8" COAX (LEASE)

ERICSSON 4449

RAYCAP RCMDC-6627-PF-48 FINAL CONFIGURATION

ERICSSON 4449

ERICSSON 8843

1-5/8" COAX

ERICSSON RRUS 12 B4

RAYCAP RCMDC-6627-PF-48

AMPHENOL WBX045T19ROOOG

EQUIPMENT TO BE REMOVED

EQUIPMENT TO BE INSTALLED

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|---|--|
| P | ROJECT INFORMATION:  |
|   | SITE NAME:<br>CANTEBURY HOUSE<br>SITE No.: 263773<br>PROJECT #: 20171625685<br>175 MARKET STREET<br>CHARLESTON SC 29401<br>CHARLESTON COUNTY<br>PLANS PREPARED B Y : |
|   |  |
|   | PHONE: 770-619-4280<br>WWWKMLEY-HORNCOM<br>SC License C00166   |
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|   | SCOPE OF WORK<br>SUMMARY   |
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